

SECTION 1

EXECUTIVE SUMMARY

1-1 INTRODUCTION

Background

The City of Manhattan Beach is a full service City with a population of 36,718. The City provides sewer services to approximately 3.9 square miles of the land located within its corporate boundaries.

The City's existing wastewater collection system is made up of a network of gravity sewers, pump stations, and force mains. The gravity system consists of approximately 81.6 miles (430,784 ft) of pipe and 2,086 manholes and cleanouts. The system also includes eight pump stations and 5,114 feet of associated forcemains.

Previous Studies and Improvements

The City's last Wastewater System Master Plan was prepared in 1994 by Kennedy/Jenks Consultants. It analyzed the wastewater collection and pumping system to provide service through a planning period that extended to the year 2010. The City's year 1990 population was approximately 32,023.

The 1994 Wastewater System Master Plan included hydraulic analysis of all pipes over 8-inches in diameter. Of those analyzed, only two pipes were identified to be hydraulically inadequate. Video inspections were completed in the "Sand" and "Tree" sections of the City and the facilities were found to be in good condition. An ongoing rehabilitation program was recommended at an estimated annual cost of \$250,000.

The pumping system was found to require the most immediate attention. It was recommended that pumps and motors be replaced at six of the primary pump stations. Replacement of the natural gas engines with dedicated generator sets; replacement of dry well ventilation; rehabilitation of pump station wiring, main breakers, starters, and disconnect switches; and conversion of analog alarm SCADA system to digital was also recommended for each station. The total pump station improvement costs were estimated at \$707,500.

Five wastewater pump stations were rehabilitated in 1998. These included Pacific Avenue Pump Station, Palm Avenue Pump Station, Meadows Avenue Pump Station, Voorhees Avenue Pump Station, and Bell Avenue Pump Station. Pumps and motors were replaced at each station. Diesel fueled generator sets were installed in the dry wells or on site. Ventilation and SCADA systems were also upgraded.

Objectives

The objective of this Wastewater Master Plan is to evaluate the City's sewer collection system to provide a framework for undertaking the construction of new and replacement facilities for the service area in an efficient and cost effective manner. As a planning document, it is general in nature and is predicated upon the best information available at this time. It will aid the City in meeting some of the requirements of the Statewide General Waste Discharge Requirements issued by the California Regional Water Quality Control Board on May 2, 2006.

1-2 STUDY AREA

Service Area

The City of Manhattan Beach (City) is located on the western edge of Los Angeles County, approximately 22 miles southwest of downtown Los Angeles. The City encompasses approximately 3.9 square miles of residential, commercial, and industrial land. It is a coastal community bounded by the approximately 2 miles of beach frontage to the west. Manhattan Beach is bordered by the City of El Segundo to the north, the Cities of Hawthorne and Redondo Beach to the east, and the Cities of Redondo Beach and Hermosa Beach to the south.

Topographical Description and Geology

The characteristic topography of Manhattan Beach is a series of peaks and valleys throughout the City. The City's highest point is located near the intersection of Sepulveda Boulevard and Longfellow Drive and reaches 235 feet above sea level. The lowest ground elevation is at 0.0 ft, along the beach frontage.

The study area can be roughly divided into two sections based on its topography and soil conditions. A sand dune ridge runs diagonally from a point on the northwest City boundary approximately 2,000 feet from the coast to a point on the southern City boundary approximately 1,000 feet east of Sepulveda Boulevard. To the west of this ridge, the soil is fine dune sand and the topography is hilly. To the east of the ridge, the soil is sandy loam and the land is flat to gently sloping.

Climate

The study area has a Mediterranean climate, enjoying plenty of sunshine throughout the year, with an average of 263 sunshine days and only 35 days with measurable precipitation annually. The period of April through November is warm to hot and dry with average high temperatures of 71 - 79°F and lows of 50 - 62°F. The average annual rainfall of about 13.19 inches occurs primarily during the winter months, between November and March.

Land Use

The City is a well planned urban community with a balance of residential, commercial, and industrial land uses. The primary land use is residential (1,422 net acres or 70.5 %). The total study area includes approximately 2,017 net acres.

Population

The City of Manhattan Beach had a population of 36,718 in 2009. With the total number of housing units at approximately 15,580 and a 3.72 percent vacancy rate, the population per household is estimated to be 2.45 (California Department of Finance, Demographic Research Unit).

1-3 CRITERIA

General

Establishing performance standards is an important part of evaluating existing wastewater collection systems, as it forms the basis for system analysis and system improvement recommendations. These standards include methodology for estimating wastewater design flows and minimum design standards for the collection system pipes, pump stations, and forcemains.

Flow Monitoring

A temporary flow monitoring study was conducted by ADS Environmental Services from December 21, 2008 to March 14, 2009 at eight locations. The flow monitoring sites were strategically selected to aid in the development of unit flow factors and calibration of the model. Sites were selected in an attempt to get a good sampling of data across the study area. At the same time, the areas tributary to each site must generate depths of flow large enough to develop accurate wastewater flows.

Unit Flow Factors

Unit flow factors utilized in this study were developed based upon the existing land uses obtained from the City's GIS, and results of the flow monitoring studies. Water use records, aerial photographs and field reviews supplemented this information.

The average daily flow recorded at each flow monitoring site was utilized in determining calibrated existing unit flow factors for each land use. The flow factors were developed in units of gallons per day per acre.

Peaking Factors

The adequacy of a sewage collection system must be based upon its ability to convey the peak flows. The temporary flow monitoring data was reviewed to develop peaking relationships at each site. As expected, these relationships varied from site to site depending upon the makeup and size of the tributary land use. Based upon the information developed, the following peaking relationships were selected for this study:

$$\text{PDWF (mgd)} = 2.35 \times \text{ADWF (mgd)}^{0.92}$$

$$\text{PWWF} = 1.35 \times \text{PDWF}$$

Where, ADWF = Average Dry Weather Flow

PDWF = Peak Dry Weather Flow

PWWF = Peak Wet Weather Flow

Sewer Design Criteria

Design criteria are established to ensure that the collection system can operate effectively under all flow conditions. Each pipe segment must convey peak wet weather flows without surcharging the system. Low flows must be conveyed at a velocity that will prevent solids from settling and blocking the system. Details of the criteria recommended for the collection system and lift stations, and service to Specific Plan and Development sub-areas are included in Section 4 of this report.

1-4 EXISTING WASTEWATER COLLECTION SYSTEM

General Description

The City of Manhattan Beach's existing sewer collection system, shown in Figure 1-1, is made up of a network of gravity sewers, pump stations, and force mains. The gravity system consists of approximately 81.6 miles (430,784 ft) of pipe and 2,086 manholes and cleanouts. The system also includes eight pump stations and 5,114 feet of associated forcemains.

The majority of the local sewers tie into one of the Los Angeles County Sanitation District (LACSD) trunk sewers crossing through the City. The sewage is then transported to LACSD's Joint Water Pollution Control Plant (JWPCP) in the City of Carson.

The sewers are primarily constructed of vitrified clay pipe with sizes ranging from 6-inches to 21-inches in diameter. Approximately 78 percent of the pipes are 8-inches in diameter. The majority of the City's sewer system was constructed between 1920 and 1960.

Los Angeles County Sanitation District (LACSD) Wastewater Collection System

Regional wastewater services are provided to the City and neighboring agencies by the Los Angeles County Sanitation District (LACSD). The City is a part of LACSD's South Bay Cities District. Regional trunk sewers collect the sewage generated in the service area and transport it to LACSD's Joint Water Pollution Control Plant (JWPCP) for treatment. The JWPCP is located in the southwest corner of City of Carson just east of I-110 freeway.

There are two primary LACSD trunk sewers within the City. The first trunk runs northwest to southeast and parallels The Strand along the beachfront. The second trunk runs west to east and is located in 26th Place, Bell Avenue, 25th Street and Marine Avenue. Pacific Avenue Pump Station, Palm Avenue Pump Station, Poinsettia Avenue Pump Station, Meadows Avenue Pump Station and Voorhees Avenue Pump Station are tributary to this trunk sewer.

1-5 PUMP STATIONS

The City of Manhattan Beach owns and operates eight (8) wastewater pump stations. Table 1-1 provides general information on each facility. Detailed description and planning level evaluation of each pump station are included in Section 6.

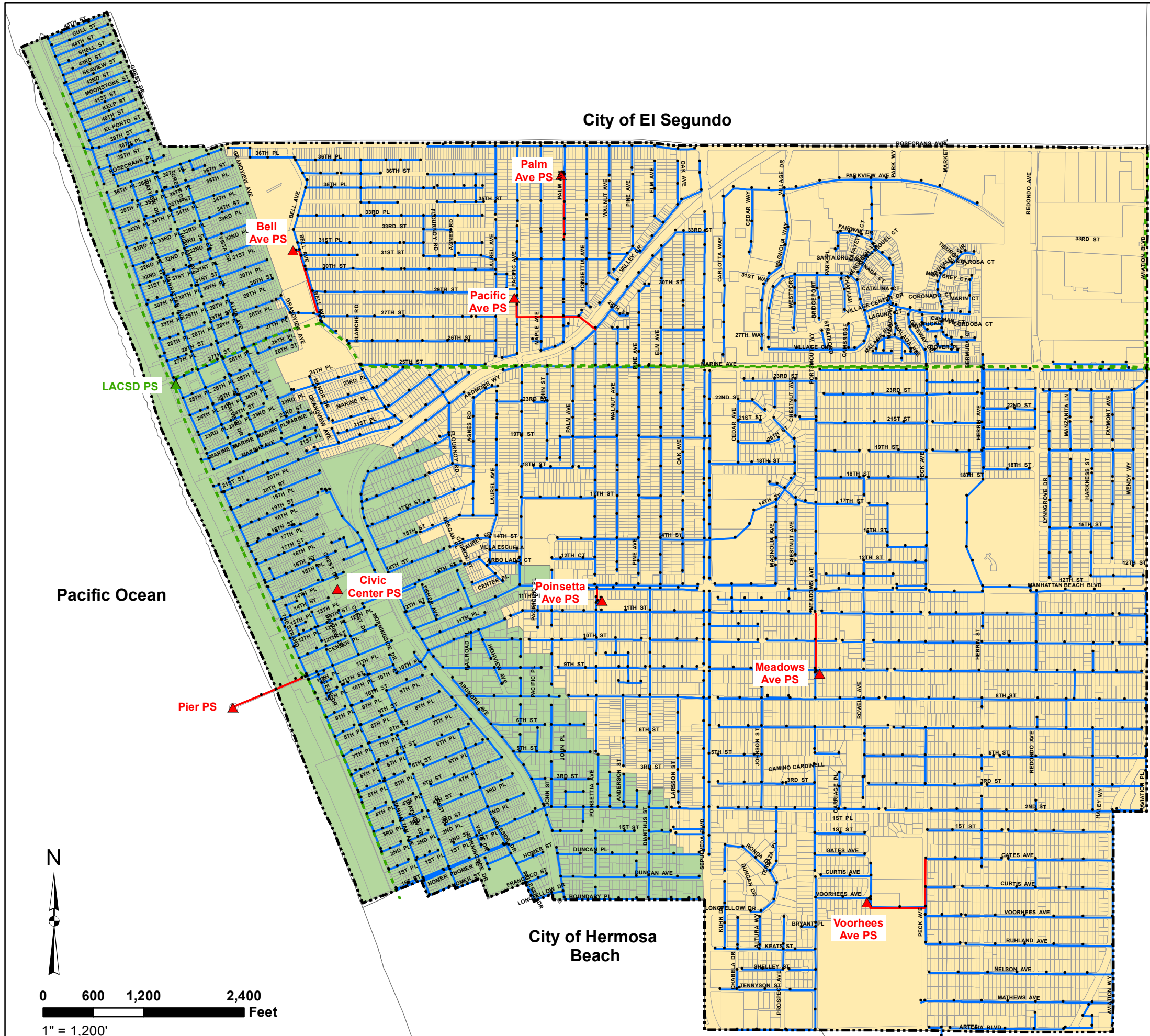
1-6 HYDRAULIC MODEL AND COLLECTION SYSTEM CAPACITY ANALYSIS

General

To perform a detailed evaluation of the capacity of the sewer collection system, it is convenient to create a mathematical model that is capable of simulating the operating characteristics of the system. The simulations for this study were performed on a Microsoft Windows based computer utilizing software designed for the analysis of sewer systems. The software selected for this study is InfoSewer. It is an ArcGIS-based computer program with the ability to perform steady state analyses of the flows in the sewer system. InfoSewer offers direct ArcGIS integration allowing GIS analysis and hydraulic modeling to exist in a single environment. The program also manages and maintains the database that stores the sewer analysis input and output results. Manning's Equation is used for depth of flow calculations in the gravity sewer pipes.

Geometric Model

As a part of this Master Plan project, the City's Wastewater GIS was developed. As-built plans were georeferenced and the wastewater facilities were then digitized. Data was collected from the as-built plans and input into the GIS. The developed Wastewater GIS served as the basis of the system geometric model. The hydraulic model includes the entire sewer system that is owned and operated by the City, from the uppermost reaches of the system to its confluence with a Los Angeles County Sanitation District (LACSD) trunk sewer or until the flow exits the City into an adjacent agency facility.



City of Hawthorne

City of Redondo Beach

City of Hermosa Beach

Legend

- Sewer Manholes
- ▲ Manhattan Beach Sewage Pump Station
- ▲ LACSD Sewage Pump Station
- Manhattan Beach Forcemain
- Manhattan Beach Sewer
- - - LACSD Trunk Sewer
- - - City Boundary
- Sewershed 1
- Sewershed 2

N

0 600 1,200 2,400 Feet

1" = 1,200'

		CITY OF MANHATTAN BEACH WASTEWATER MASTER PLAN
		Existing Wastewater System Figure 1-1
PROJECT NO: 1640901.10 DATE: September 2009		

**Table 1-1
Pump Stations**

General Pump Station Information										Pump Specifications					Motor Specifications			Estimated Flows			
Pump Station	Location	Date of Cons.	Plan No.	Area Served (Acres)	Force Main Dia (in)	Force Main Length (ft)	Force Main Material	Type	Wet Well Dimensions	Generator Set	Number of Pumps	Pump Mfg	Pump Type	Pump Model	Estimated Flow Capacity (gpm)	RPM	Phase	HP	ADWF (gpm)	PDWF (gpm)	PWWF (gpm)
Bell Avenue	Bell Ave between 31st St and 31st Pl	1938	S-60 S-150 S-230	78	6	900	CIP	Wetwell/ Drywell	12' x 9.5' x ~7'	Diesel Fueled	2	Flygt	submersible, non-clog	CT3102	263	1750	3	5	70	198	278
Palm Avenue	Palm Ave south of Rosecrans Ave	1953	S-149 S-230	39	6	775	CIP	Wetwell/ Drywell	10' x 4' x ~11.5'	Diesel Fueled	2	Flygt	submersible, non-clog	CT3127	174	1740	3	10	38	114	159
Pacific Avenue	Pacific Ave south of 31st St	1953	S-149	103	6	1,225	CIP	Wetwell/ Drywell	10' x 4' x ~11.5'	Diesel Fueled	2	Flygt	submersible, non-clog	CT3127	304	1740	3	10	98	269	376
Poinsettia Avenue	Poinsettia Ave south of Manhattan Beach Blvd	1949	S-227 S-145	25	4	163	CIP	Wetwell/ Drywell	18.6 ft ² x 6'-10"	Diesel Fueled	2	Flygt	submersible, non-clog	CT3102	136	NA	3	NA	19	60	85
Meadows Avenue	Meadows Ave south of 9th St	1953	S-148 S-230	99	6	730	CIP	Wetwell/ Drywell	10' x 4' x ~11.5'	Diesel Fueled	2	Flygt	submersible, non-clog	CT3127	304	1740	3	10	77	216	303
Voorhees Avenue	Voorhees Ave at Rowell Ave	1953	S-148 S-230	103	6	1,321	CIP	Wetwell/ Drywell	10' x 4' x ~11.5'	Diesel Fueled	2	Flygt	submersible, non-clog	CT3127	227	1740	3	10	90	250	350
Civic Center	Highland Ave south of 15th St	1992	-	Civic Center Bulding	-	-	-	Submersible	-	-	2	-	-	-	-	-	-	-	-	-	-
Pier	End of Manhattan Beach Pier	1992	-	Pier Restaurant	4	-	CIP	Submersible	4' x 4' x 4'	-	2	-	submersible	-	-	-	-	3	-	-	-

Land Use

The parcel layer, which provides the City's 2003 General Plan land use information, was used as the land use base map. Since the City's service area is mostly developed, the hydraulic analyses were conducted assuming fully developed and occupied tributary areas.

Tributary Areas

Polygons were created around individual sewer manholes to define the tributary area to each manhole. Most manholes have a tributary area assigned to it unless there are multiple manholes in the same area. Approximately 1,530 polygons were created for the existing and ultimate system analysis.

Model Loads

For each tributary area, the existing land uses and the developed unit flow factors were utilized to apply the average loads to the hydraulic model.

Peak dry weather flows are calculated in the model by a user defined relationship. The peaking formula used in the City's sewer model is as follows:

$$\text{PDWF (cfs)} = 2.35 \times \text{ADWF(cfs)}^{0.92}$$

Pumped Flows

There are two separate analysis methods that can be used on the City's sewer collection system to evaluate the effect of pumped flows downstream of the existing 8 pump stations. A description of each of the analysis methods is as follows:

Method 1: The average tributary flow to each pump station was transferred to the manholes located at the discharge end of the respective forcemains. The total average flow was peaked and the downstream sewer depth to diameter ratios were evaluated based on the calculated peak dry weather flows or

$$Q_{pdw} = (2.35 \times Q_{adw}^{0.92}) \text{ (in mgd)}$$

Method 2: The pump capacity of each pump station was transferred to the manholes located at the discharge end of the respective forcemains. The pump capacities were not peaked, but added to the peak dry weather flow at each manhole located downstream of the pump station. The downstream sewer depth to diameter ratios were evaluated based on the calculated peak dry weather flows plus the pump capacities (Q_{pump}) or

$$Q_{pdw} = (2.35 \times Q_{adw}^{0.92}) + Q_{\text{pump}} \text{ (in mgd)}$$

Flow monitoring results have shown that pump discharge becomes more and more attenuated the further downstream in the system the monitor is placed. Often times, only the first few reaches located immediately downstream of the discharge point will experience a rush of flow that reflects the pump capacity. In this study, the pump capacity was considered if flow monitoring results showed an influence from the pump station discharge. For the pipes located a far distance from the pump discharge point, the calculated peak dry weather flow based on land use and unit flow factors was utilized for analysis.

Hydraulic Analysis Results

The depth to diameter ratio exceeded the established criteria of 0.64 at the following locations:

1. Bell Avenue at 25th Street (Pipe ID 15084-070L4)
10" diameter sewer with peak d/D>1.00
No pump station influence

As-built plans show a slope of 0.00 and a length of 10 feet. The downstream manhole 070L4 is a LACSD manhole. It is recommended that the slope of this sewer be verified.

2. Pacific Avenue north of 27th Street (Pipe ID 08077-08082)
8" diameter sewer with peak d/D=0.66 with no pump station influence
8" diameter sewer with peak d/D>1.00 with pump station influence from Palm Avenue PS (see Table 7-2 and description below)

A few reaches downstream of the Poinsettia Avenue Pump Station, Palm Avenue Pump Station, and Bell Avenue Pump Station discharge locations resulted in d/D ratios greater than 0.64 when the pump capacities were implemented (Method 2). These locations are listed in Table 1-2. It is recommended that flow monitoring be conducted in these areas to verify the d/D ratios prior to implementing any replacement projects.

**Table 1-2
Pipes with Model Calculated Capacity Deficiencies Downstream of Pump Discharge Points**

Pipe ID	U/S MH ID	D/S MH ID	Dia (in)	Length (ft)	Slope	ADWF (mgd)	Pumped Flow (mgd)	PDWF (mgd)	PDWF Vel (ft/s)	PDWF d/D	PDWF Depth (ft)	Full Flow (mgd)	Comments
06173-06174	06-173	06-174	8	192	0.0040	0.0019	0.7363	0.7437	3.30	1.00	0.67	0.4959	D/S of Poinsettia PS
06174-06175	06-174	06-175	8	254	0.0040	0.0028	0.7363	0.7469	3.31	1.00	0.67	0.4962	D/S of Poinsettia PS
06175-06176	06-175	06-176	8	190	0.0136	0.0033	0.7363	0.7485	4.51	0.69	0.46	0.9125	D/S of Poinsettia PS
08037-08052	08-037	08-052	8	262	0.0040	0.0446	0.4085	0.5430	2.41	1.00	0.67	0.4957	D/S of Palm PS
08052-08055	08-052	08-055	8	281	0.0040	0.0583	0.4085	0.5805	2.57	1.00	0.67	0.4944	D/S of Palm PS
08055-08077	08-055	08-077	8	350	0.0040	0.0637	0.4085	0.5951	2.64	1.00	0.67	0.4953	D/S of Palm PS
08077-08082	08-077	08-082	8	110	0.0040	0.0858	0.4085	0.6538	2.90	1.00	0.67	0.4951	D/S of Palm PS
16019-16022	16-019	16-022	8	68	0.0099	0.0131	0.5728	0.6164	3.84	0.67	0.45	0.7809	D/S of Bell PS
16022-070L3	16-022	07-0L3	10	18	0.0028	0.0135	0.5728	0.6176	2.38	0.69	0.58	0.7504	D/S of Bell PS
				Total	1,725								

1-7 COLLECTION SYSTEM CONDITION ASSESSMENT

General

Condition of the collection system depends on many factors, including construction quality, and maintenance practices, surrounding soil and groundwater conditions, flow quantity and velocity, and constituents in sewage. The best available information on the system condition is provided by the closed circuit television (CCTV) inspection reports and recordings and the knowledge of the maintenance staff.

The City of Manhattan Beach (City) established a program to begin CCTV inspections of its approximately 86 mile long sewer collection system. The scope of the Master Plan included inspections of 227,714 feet (43.1 miles) of pipe or 52.9 percent of the total system.

Closed Circuit Television (CCTV) Inspections

Empire Pipe Cleaning and Equipment, Inc. (Empire) performed video inspection work on approximately 222,714 feet of pipe between October 2008 and August 2009. National Association of Sewer Service Companies (NASSCO) Pipeline Assessment and Certification Program (PACP) coding procedures formed the basis of the inspection work and reports. Each inspection report lists the service connections and deficiencies by location in the inspected pipe. Photographs of the identified deficiencies are included in the inspection reports.

All data was compiled into one summary database, which can be found in Appendix 5. The database includes a summary count of the deficiencies identified in the CCTV written reports for the 1,200 reaches were inspected.

Review of Representative CCTV Recordings

The Inspection Report Database Summary was used in selecting the recordings to be reviewed in detail. The pipe reaches selected for detailed review were those that showed the most severe structural problems and multiple deficiencies, such as deformed pipe, hole in pipe, broken pipe, large offset joint, large obstacles, multiple deficiencies such as fractures, cracks, roots, deposits, obstructions, sags, camera underwater, and survey abandoned, and ball roots, as well as severe operation and maintenance issues. Several reaches without listed deficiencies were selected in order to develop insight into the overall condition of the CCTV inspected system. Recordings for 264 reaches and 53,789 feet of pipe (23.9% of total inspected) were selected and reviewed in detail.

Condition Grading

The PACP condition grading system was used to assign a condition rating for structural defects and operation and maintenance defects for each reach of pipe. The rating provides the ability to quantitatively measure the difference in pipe condition between one inspection and subsequent inspections, and to prioritize among different pipe segments. A grade of 1 to 5 is assigned to each defect based on potential for further deterioration or pipe failure. Pipe failure is defined as when it can no longer convey the design capacity. The grades are as follows:

- 5 – Immediate Attention Defects requiring immediate attention
- 4 – Poor Severe defects that will become Grade 5 defects within the foreseeable future
- 3 – Fair Moderate defects that will continue to deteriorate
- 2 – Good Defects that have not begun to deteriorate
- 1 – Excellent Minor defects

The problems identified most often were fine roots (835 reaches, 22% of total), cracks (563 reaches, 15% of total) and joint offset medium (437 reaches, 12% of total).

Replacement and Rehabilitation Priorities

The purpose of CCTV inspections is to determine the condition of the City's existing gravity sewers, and formulate a rehabilitation plan for the defective sewers. The defect scores and indexes provide a good indication as to which pipes are in poor condition, but cannot be relied upon solely to prioritize improvement projects. The priorities are selected primarily with consideration of the health and safety of the public and protection of the environment by minimizing the possibility of sanitary sewer overflows and leakage. The pipe capacity, location of particular defects, and the tributary areas/wastewater flow rates are other considerations used in formulating the final capital improvement project priorities.

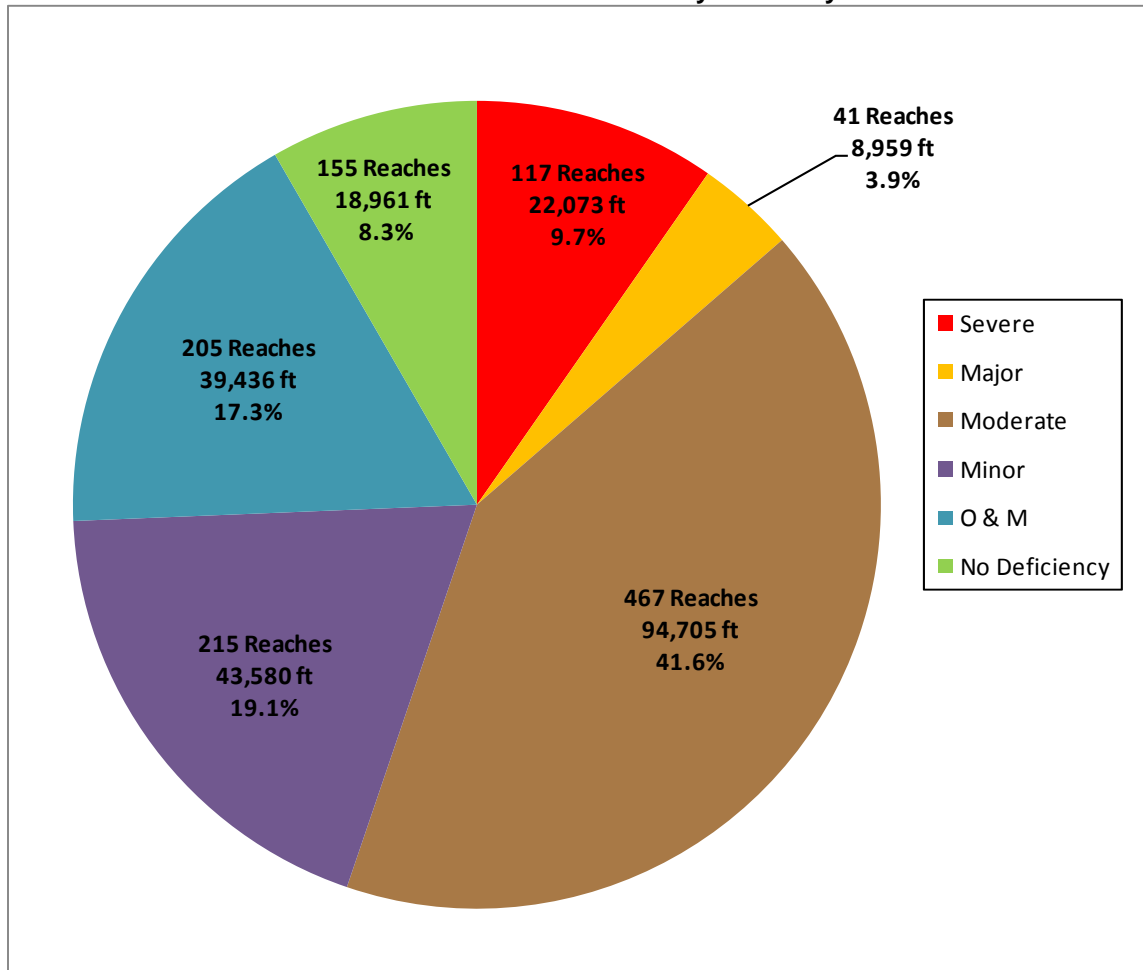
The initial priorities for improvements to the sewers are based on the severity of the pipe defects. The six (6) categories utilized in this report are as follows:

- a. Severe Condition – This category primarily includes structural defects of deformed pipe, hole in pipe, broken pipe, and large joint offsets.
- b. Major Condition – This category primarily includes structural defects of multiple fractures, medium joint offsets and major sags. Pipes with a large number of cracks are also included.
- c. Moderate Condition – Pipes in this category have fractures, cracks, small and medium joint offsets, and sags.
- d. Minor Condition – Pipes in this category have slight sags, cracks, and small joint offsets.
- e. O&M – This condition is for operational and maintenance problems and construction feature defects. There are no structural defects.
- f. No Defects – This condition is for the pipe with no structural, operation and maintenance or construction feature defects.

Figure 1-2 shows the distribution of the condition priorities assigned the pipes with completed inspections.

The City of Manhattan Beach will address the “Severe” and “Major” collection system deficiencies. The collection system construction estimates are based upon replacement at \$45 per diameter inch per foot of pipe. Implementation cost is determined by adding 35 percent of construction cost to cover engineering, inspection, and administration. The total estimated cost of upgrading the sewers with severe or major condition priorities is \$15,330,000. This is based on the assessment of 43.1 miles of pipe or 52.9 percent of the total gravity sewer system. Assuming that similar deficiencies will be identified when the remainder of the system is CCTV inspected, the total cost for rehabilitation and replacement of the remaining gravity sewer pipes is estimated at approximately \$13,600,000.

**Figure 1-2
Condition Assessment Priority Summary**



Follow-up CCTV Inspection and Condition Assessment Program

- a. Portions of the system rated to be in **Severe Structural Deficiency** condition will be inspected **annually** and evaluated to determine if immediate corrective action is needed.
- b. Portions of the system rated to be in **Major Structural Deficiency** condition will be CCTV inspected and evaluated once every **three (3) years**
- c. Portions of the system rated to be in **Moderate Structural Deficiency** condition will be CCTV inspected and evaluated once every **five (5) years**
- d. Portions of the system rated to be in **Minor Structural Deficiency** condition will be CCTV inspected and evaluated once every **ten (10) years**
- e. Portions of the system with **no structural deficiencies** will be CCTV inspected and evaluated once every **ten (10) years**

- f. Portions of the system with **Operational and Maintenance** deficiencies, except the **Hot Spots**, will be CCTV inspected and evaluated once every **four (4) years**.
- g. **Hot Spots**, except siphons, will be CCTV inspected and evaluated **before and after each maintenance activity and cleaning for one year** to establish the appropriateness of the method, and then **annually**.

As structural deficiency mitigation projects are implemented, their condition will be reclassified, and they will be included in the appropriate category for follow up CCTV inspection and condition assessment work.

Manhole Inspections and Assessments

The condition of the manholes associated with the 52.9 miles of gravity pipes inspected was also assessed. A total of 1,075 manholes were inspected and assessed. Each element was rated as good, fair, poor, or failing. Signs of debris, grease, vermin, and odors were also noted in the inspection reports. The results of the inspections are summarized in Table 1-3.

**Table 1-3
Manhole Elements Condition Summary**

Condition	Manhole Cover	Frame	Cone	Barrel / Wall	Rungs	Bench	Channel	Debris	Grease	Vermin	Odor
Good	1070	1068	1030	1048	612	1022	960				
Fair		7	37	22	155	38	95				
Poor			7	4	150	15	20				
Failing			1	1	67						
Broken	2										
Cracked	3										
Not Applicable					91						
Yes								15		55	3
No								1060	1075	1020	1072

The total estimated cost of upgrading the manholes with poor conditions is \$316,000. This is based on the assessment 1,075 manholes or 53 percent of all the manholes. The total number of manholes in the sewer system is estimated at 2,031. Assuming that similar deficiencies will be identified when the remainder of the system is CCTV inspected, the total remaining cost for rehabilitation and replacement of the manholes in poor condition is estimated at approximately \$280,000.

1-8 OPERATION AND MAINTENANCE

CCTV Inspection Data

The operation and maintenance conditions identified through CCTV inspections include ball roots, grease, sags, deposits, obstacles, and infiltration, respectively. These conditions are the cause of many of the

frequent maintenance areas (hot spots). Reduction in cross sectional pipe area is a potential cause for back ups and sewer overflows. Accordingly, the highest priority is given to those reaches identified with ball roots.

The recommendations provided should be used in updating the operation and maintenance of the system, the Hot Spot maintenance program, the cleaning program, as well as the Preventative Maintenance Program.

'Hot Spots'

Hot Spots are areas of the system with reoccurring problems that require maintenance and cleaning on a quarterly basis minimum. Currently, there are 76 reaches with a total length of 10,796 feet that are considered to be Hot Spots in the City's system. Operations staff reports that the causes of the hot spots are grease and roots.

Sanitary Sewer Overflow (SSO) History

There were a total of 19 sanitary sewer overflows responded to by City crews between January 2007 and June 2009. The total number of reported spills that occurred on City property over the past three years is as follows:

7 spills in 2007 (8.6 spills per 100 miles)

3 spills in 2008 (3.7 spills per 100)

1 spill in 2009 (1.2 spills per 100) – *up to September 2009*

A sewer collection system with less than three (3) spills from the publicly owned system (excludes private property spills that do not result from a blockage in the public system) per 100 miles per year is considered an adequate system. For the City's sewer system (81.6 miles), this is an average of about 2.4 (3 x 0.816) spills per year.

Maintenance Program

A comprehensive maintenance program is an important tool in assuring reliable system operation. This not only includes regular inspections and preventative maintenance, but also good record keeping. Accurate records are the backbone of any maintenance operation. They can be used for many purposes including: scheduling regular maintenance activities; allocating manpower; budgeting; pinpointing persistent problems; tracking equipment performance and maintenance history; and the identification of equipment which may be showing signs of failure.

Preventative Maintenance

Preventative maintenance is a crucial element of the maintenance program. The preventative maintenance program (PMP) consists of cleaning, inspection, condition assessment, and rehabilitation tasks.

Sewer inspection includes CCTV inspection and condition assessment of the collection system, visual inspection of manholes and their flow channels, ground surface inspection of rights of way and easements, and odor and corrosion monitoring. Condition assessment includes, review of the inspection data, and formulation of maintenance, rehabilitation, and replacement projects. Following the completion of the initial CCTV inspection program, the City should develop a continuing inspection plan based upon the knowledge

gained from the initial program. Hot Spots, except siphons, should be CCTV inspected and evaluated before and after each maintenance activity and cleaning for one year to establish the appropriateness of the method, and then annually. Each spill site must be CCTV inspected to pinpoint the cause of the spill, and implementation of corrective measures for preventing repeat spills.

Preventative maintenance activities that the City does currently conduct include the following:

- Clean entire gravity system twice per year
- Clean hot spots twice per month
- Inspect and skim wet wells twice per week
- Vacuum out wet wells once per year
- Inspect pump stations once per week by Utilities Electrician and Sewer Maintenance Worker
- Diagnose pump station motor control centers annually for operational efficiency and safety by the Southern California Edison Company
- Catalogue and track tasks by work orders in a maintenance management system
- Perform routine mechanical root removal in areas of known root intrusion
- Start test the emergency power systems at the pump stations weekly
- Service generators bi-annually by Power Plus, Inc.

Maintenance Staff and Training

The City currently has about 81.6 miles of pipe. In order to comply with the WDR requirements and the City's regular preventative maintenance program, the City must quantify the number of employees and equipment necessary to perform these tasks.

The City's current staffing for the wastewater collections system maintenance includes 5 employees, one supervisor and four field staff members. Training of these staff members is as follows:

- a. Initial training in proper operation and maintenance of all major wastewater equipment and facilities by the Wastewater Supervisor
- b. Critical equipment training is facilitated by outside contractors in areas of fork lift operation, backhoe and bob-cat operation, and crane operation
- c. Periodic training in confined space entry, trench shoring, traffic control, heat illness, and hazardous materials management
- d. Task proficiency is required for all positions and training records are maintained to monitor completed classes
- e. Disaster training exercises are conducted twice per year
- f. Staff is also trained in Best Management Practices of the Fats, Oils, and Grease (FOG) Control Ordinance and is instructed to report violations

1-9 CAPITAL IMPROVEMENT PROGRAM

General Description

The primary goal of the Capital Improvement Program (CIP) is to provide the City of Manhattan Beach with a long-range planning tool for implementing its sewer infrastructure improvements in an orderly manner and a basis for financing of these improvements. To accomplish this goal, the program is phased based upon the implementation cost of the facilities, the quantity of work the City can reasonably administer each year, and the funds available for these projects.

Capital Improvement Project Priorities

The capital improvement projects were selected primarily with consideration of the health and safety of the public and protection of the environment by minimizing the possibility of overflows. The projects that will eliminate the capacity deficiencies in the gravity collection system are prioritized based upon the hydraulic analyses conducted during this study. As the City completes CCTV inspection of the system, severe and major defects identified should be incorporated into the CIP and addressed. When the CCTV inspection is completed and a full condition assessment has been conducted, the capital improvement project priorities should be reevaluated.

Collection System Capacity Improvement Projects

The collection system capacity improvement projects include the areas identified with a capacity deficiency in the hydraulic model when pump capacities were implemented. It is recommended that the identified locations be flow monitored to verify the d/D ratios prior to implementing any replacement projects. Operations staff has not indicated that these areas are a problem. Therefore, until the deficiencies are verified in the field, these projects are considered low in priority.

Collection System Condition Improvement Projects

The condition improvement projects are prioritized solely on the condition of the pipe as determined from reviews of the CCTV recordings. The condition deficiencies with critical structural damage and severe obstructions were given the highest priority. Sewer pipes with conditions categorized as "Severe" or "Major" and manholes categorized as in poor condition are included in the recommended improvements.

The planning level recommendations are based upon the ranking and pipe defects from the CCTV inspection reports, and reviews of recordings. It may be possible to reline, repair or perform root treatment on some of the existing gravity pipes, in lieu of replacing them. Actual improvements should be designed based upon further detailed reviews of each recording, taking into consideration other factors such as location, age, capacity of the pipe, existing utilities, and concurrent infrastructure construction projects.

The useful life gained from replacing the deficient facilities will be longer than repairs and relining projects. Root treatment is usually a temporary solution. Unless the source is removed, it is likely that the roots will get thicker as time passes and the root intrusion will continue until the pipe is replaced.

Pump Station Improvements

The recommended pump station capital improvement projects have been based upon condition assessment of each facility, capacity analysis, and conformance with the adopted criteria. The implementation priorities should be based upon the likelihood of a failure that may result in a spill, the volume of spill, and its impact on the public and the environment. The condition assessment and analysis results are described in detail Section 6 for each pump station.

Capital Improvement Program

The Capital Improvement Program is developed based upon the results of the hydraulic analyses and the established priorities. The recommended improvement project locations are illustrated on Figure 1-3.

Gravity collection system projects are listed in Table 1-4 and Table 1-5 by priority, along with cost estimates. The cost estimates presented in Table 1-4 and Table 1-5 reflect replacement of the existing facilities. Replacement costs are generally more conservative and will therefore allow the City more flexibility for each project. The pipeline construction costs are based upon \$45 / diameter inch / ft. Preliminary design studies should be conducted utilizing detailed utility information to identify and evaluate project alternatives such as parallel pipes and/or diversions prior to final design. When sewers are replaced, they should be relocated into the walk ways or right-of-ways as much as possible if they are currently in a location that cannot be easily accessed.

The City of Manhattan Beach is largely occupied and there are many existing utilities to consider. Therefore, the costs of replacing sewer facilities will be generally higher than in an area that is undeveloped. The total costs shown in Table 1-4 and Table 1-5 include engineering, administration and contingency costs, estimated at 35% of the construction cost.

The manhole rehabilitation and replacement projects are listed in Table 1-6. The pump station and forcemain improvement projects are listed in Table 1-7.

The recommended projects have been based upon the best information currently available. It should be updated as new information becomes available from sources such as CCTV inspections and from maintenance crew observations. The project priorities may be revised to correspond to changed conditions, such as impending facility failures, or to take advantage of concurrent construction such as street paving projects or adjacent infrastructure work.

Some of the projects recommended are small and it may not be feasible to implement them as a single project. Therefore, several projects should be combined and bid as a package. Some of the projects may be broken down into smaller components to fit the City's budgetary and other obligations.

The total Wastewater Capital Improvement Program is shown in Table 1-8. The total CIP costs are estimated at \$42,042,640.

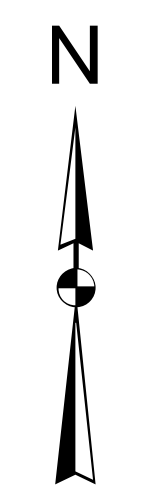
City of El Segundo

City of Hawthorne

City of Redondo Beach

City of Hermosa Beach


Pacific Ocean




0 400 800 1,600 Feet
1" = 400'

Legend

- Sewer Manholes
- ▲ Manhattan Beach Sewage Pump Station
- ▲ LACSD Sewage Pump Station
- Manhattan Beach Forcemain
- Manhattan Beach Sewer
- LACSD Trunk Sewer
- City Boundary
- M-## Manhole Condition Deficiency Project Location and ID
- G-## Severe Condition Deficiency Project Location and ID
- G-## Major Condition Deficiency Project Location and ID
- C-## Capacity Deficiency Project Location and ID
- P-## Pump Station and/or Forcemain Project Location and ID



CITY OF MANHATTAN BEACH
WASTEWATER MASTER PLAN
Capital Improvement Projects
Figure 1-3



PROJECT NO: 1640901.10
DATE: September 2010



**Table 1-4
Collection System Capacity Improvement Projects**

Project No.	Pipe ID	U/S MH ID	D/S MH ID	Existing Dia (in)	Length (ft)	Slope	ADWF (mgd)	Pumped Flow (mgd)	PDWF (mgd)	Comments	Replacement Size (in)	Unit Cost (\$)	Construction Cost (\$)	Eng. & Admin. Cost (\$)	Project Cost (\$)
C-1	06173-06174	06-173	06-174	8	192	0.0040	0.0019	0.7363	0.7437	D/S of Poinsettia PS	15	675	429,300	150,255	579,555
	06174-06175	06-174	06-175	8	254	0.0040	0.0028	0.7363	0.7469						
	06175-06176	06-175	06-176	8	190	0.0136	0.0033	0.7363	0.7485						
C-2	08037-08052	08-037	08-052	8	262	0.0040	0.0446	0.4085	0.5430	D/S of Palm PS	12	540	541,658	189,580	731,238
	08052-08055	08-052	08-055	8	281	0.0040	0.0583	0.4085	0.5805						
	08055-08077	08-055	08-077	8	350	0.0040	0.0637	0.4085	0.5951						
	08077-08082	08-077	08-082	8	110	0.0040	0.0858	0.4085	0.6538						
C-3	16019-16022	16-019	16-022	8	68	0.0099	0.0131	0.5728	0.6164	D/S of Bell PS	12	540	46,591	16,307	62,898
	16022-070L3	16-022	07-0L3	10	18	0.0028	0.0135	0.5728	0.6176						
				Total	1,725									Total	1,373,691

**Table 1-6
Manhole Rehabilitation and Replacement Projects**

Project No.	Inspection Phase	Session ID	Street	Manhole	Inspection Date	Surface Condition	Manhole Cover	Frame	Cone	Barrel / Wall	Rungs	Bench	Channel	Debris	Grease	Vermin	Odor	Priority Score	Condition Ranking	Comment	Recommendation	Project Cost (\$)
M-1	1	25	Highland Ave	19-015	1/12/09	Pavement - Concrete Collar	Good	Good	Failing	Failing	Good	Good	Good	No	No	No	No	24	1	Fractures in cone and barrel. Fracture seen in street pavement surrounding manhole cover.	Replace	30,000
M-2	1	28	1st St	01-080	11/21/08	Pavement - Concrete Collar	Good	Good	Poor	Poor	Failing	Poor	Poor	No	No	0	No	22	2	Missing mortar. Corrosion at bench and channel.	Line manhole	10,000
M-3	1	74	Duncan Ave	10-039	12/19/08	Pavement - Concrete Collar	Good	Good	Good	Good	Failing	Poor	Poor	No	No	No	No	18	3	Corrosion at bench and channel.	Line manhole	10,000
M-4	1	90	Gates Ave	01-072	11/20/08	Pavement - Concrete Collar	Good	Good	Poor	Fair	Poor	Poor	Poor	No	Yes	0	Yes	15	4	Corrosion at bench and channel.	Line manhole	10,000
M-5	1	51	Curtis Ave	05-052	12/2/08	Pavement - Concrete Collar	Broken	Good	Good	Good	Poor	Poor	Poor	No	No	No	No	14	5	Broken manhole cover. Corrosion at bench and channel.	Replace manhole cover and line	13,500
M-6	1	36	2nd St	01-086	11/24/08	Pavement - Concrete Collar	Good	Good	Good	Good	Poor	Poor	Poor	Yes	No	0	No	11	6	Corrosion at bench and channel.	Line manhole	10,000
M-7	1	109	18th St	06-244	12/5/08	Pavement - Concrete Collar	Good	Good	Good	Good	Poor	Poor	Poor	No	No	No	No	10	7	Corrosion at bench and channel.	Line manhole	10,000
M-8	1	23	8th St	04-054	12/1/08	Pavement - Concrete Collar	Good	Good	Good	Good	Poor	Poor	Poor	No	No	No	No	10	8	Corrosion at bench and channel.	Line manhole	10,000
M-9	1	50	Meadows Ave	05-023	12/2/08	Pavement - Concrete Collar	Good	Good	Good	Good	Poor	Poor	Poor	No	No	No	No	10	9	Corrosion at bench and channel. Not a smooth transition.	Line manhole	10,000
M-10	1	130	14th St	06-191	12/8/08	Pavement - Concrete Collar	Good	Good	Good	Good	Poor	Poor	Poor	No	No	No	No	10	10	Corrosion at bench and channel.	Line manhole	10,000
M-11	1	291	Cedar Way	25-012	1/22/09	Pavement - Concrete Collar	Good	Good	Poor	Poor	Poor	Good	Good	No	No	No	No	10	11	Corrosion and cracking in cone and wall	Line manhole	10,000
M-12	1	293	Village Dr	25-025	1/22/09	Pavement - Concrete Collar	Good	Good	Poor	Poor	Poor	Good	Good	No	No	No	No	10	12	Corrosion in cone and barrel.	Line manhole	10,000
M-13	1	53	Rowell Ave	05-055	12/2/08	Pavement - Concrete Collar	Broken	Good	Good	Good	Fair	Good	Good	No	No	No	No	9	13	Broken manhole cover.	Replace manhole cover	3,500
M-14	1	47	Tennyson St	05-020	12/2/08	Pavement - Concrete Collar	Good	Good	Good	Good	Poor	Fair	Poor	No	No	No	No	9	14	Corrosion at channel.	Line manhole	10,000
M-15	1	60	Voorhees Ave	05-005	12/2/08	Pavement - Concrete Collar	Good	Good	Good	Good	Poor	Fair	Poor	No	No	No	No	9	15	Corrosion at channel.	Line manhole	10,000
M-16	1	181	Flournoy Rd	17-057	1/8/09	Pavement - Concrete Collar	Good	Good	Good	Good	Fair	Poor	Poor	No	No	No	No	9	16	Corrosion at bench and channel.	Line manhole	10,000
M-17	1	46	11th St	09-028	12/18/08	Pavement - Concrete Collar	Good	Good	Good	Good	Good	Poor	Poor	No	No	Yes	No	9	17	Corrosion at bench and channel.	Line manhole	10,000
M-18	2	235	The Strand	12-030A	7/20/09	Pavement - Concrete Collar	Good	Good	Good	Poor	Poor	Good	Fair	No	No	No	No	9	18	Missing mortar.	Repair mortar	6,000
M-19	1	80	14th St	06-050	12/4/08	Pavement - Concrete Collar	Good	Good	Good	Good	Good	Poor	Poor	No	No	No	No	8	19	Corrosion at bench and channel.	Line manhole	10,000
M-20	1	131	14th St	06-077	12/8/08	Pavement - Concrete Collar	Good	Good	Good	Good	Good	Poor	Poor	No	No	No	No	8	20	Corrosion at bench and channel.	Line manhole	10,000
M-21	1	178	31st St	17-053	1/8/09	Pavement - Concrete Collar	Good	Good	Good	Good	Good	Poor	Poor	No	No	No	No	8	21	Corrosion at channel.	Line manhole	10,000
M-22	1	99	Manhattan Ave	11-116A	12/22/08	Pavement - Concrete Collar	Good	Good	Poor	Good	Poor	Good	Good	No	No	No	No	8	22	Cracks in cone.	Line manhole	10,000
M-23	3	25	35th Pl	17-049	8/11/09	Pavement - Concrete Collar	Good	Good	Good	Good	Fair	Fair	Poor	No	No	0	No	8	23	Corrosion at bench and channel.	Line manhole	10,000
M-24	1	284	The Strand	14-064	1/21/09	Pavement - Concrete Collar	Cracked	Good	Good	Good	N/A	Good	Good	No	No	No	No	7	24	Cracked manhole cover.	Replace manhole cover	3,500
M-25	1	119	19th St	06-030	12/5/08	Pavement - Concrete Collar	Good	Good	Good	Good	Good	Fair	Poor	No	No	No	No	7	25	Corrosion at channel.	Line manhole	10,000
M-26	1	205	Blanche Rd	17-040	1/9/09	Pavement - Concrete Collar	Good	Good	Good	Good	Fair	Good	Poor	No	No	No	No	7	26	Corrosion at channel.	Line manhole	10,000
M-27	1	26	1st St	01-061	11/21/08	Pavement - Concrete Collar	Good	Good	Good	Good	Fair	Good	Poor	No	No	0	No	7	27	Corrosion at channel.	Line manhole	10,000
M-28	1	143	Marine Ave	15-053	1/7/09	Pavement - Concrete Collar	Good	Good	Poor	Good	Fair	Good	Good	No	No	No	No	7	28	Missing mortar and possibly bricks.	Repair mortar	6,000
M-29	1	87	Highview Ave	11-040	12/22/08	Pavement - Concrete Collar	Good	Good	Good	Good	Poor	Good	Good	Yes	No	No	No	7	29	Cracks in cone.	Line manhole	10,000
M-30	2	191	1st St	12-026	7/16/09	Pavement - Concrete Collar	Good	Good	Good	Good	Good	Poor	Fair	No	No	No	No	7	30	Corrosion at bench and channel.	Line manhole	10,000
M-31	2	278	3rd St	10-139	7/22/09	Pavement - Concrete Collar	Cracked	Good	Good	Good	Good	Good	Good	No	No	No	No	7	31	Cracked manhole cover.	Replace manhole cover	3,500
M-32	1	100	Manhattan Ave	11-117	12/22/08	Pavement - Concrete Collar	Good	Good	Poor	Good	Good	Good	Good	No	No	No	No	6	32	Cracks in cone.	Line manhole	10,000
Total																					316,000	

**Table 1-7
Pump Station and Force Main Improvement Projects**

Project No.	Project Description	Date of Construction for Existing Facility	Justification	Recommended Facilities				Unit Cost (\$)	Construction Cost (\$)	Eng. & Admin. Cost (\$)	Total Project Cost (\$)
				Firm Capacity (gpm)	Volume (gal)	Pipe Size (in)	Pipe Length (ft)				
P-1	Replace Poinsettia PS	1949	Condition / Criteria	150					2,000,000	700,000	2,700,000
	Replace Poinsettia PS Forcemain	1949	Age / Condition			4	163	300	49,000	18,000	67,000
P-2	Replace Pier PS Forcemain	1935	Age / Condition			4	900	400	360,000	126,000	486,000
P-3	Upgrade Pacific PS	1953	Criteria	400					400,000	140,000	540,000
	Replace Pacific PS Forcemain	1953	Age / Condition			6	1,225	240	294,000	102,900	396,900
P-4	Upgrade Voorhees PS	1953	Criteria	350					400,000	140,000	540,000
	Replace Voorhees PS Forcemain	1953	Age / Condition			6	930	240	223,200	78,120	301,320
P-5	Upgrade Meadows PS	1953	Criteria	310					400,000	140,000	540,000
	Replace Meadows PS Forcemain	1953	Age / Condition			6	730	240	175,200	61,320	236,520
P-6	Upgrade Bell Pump Station	1938	Criteria	300					400,000	140,000	540,000
	Replace Bell PS Forcemain	1938	Age / Condition			6	900	240	216,000	75,600	291,600
P-7	Replace Palm PS Forcemain		Age / Condition			4	775	240	186,000	65,100	251,100
P-8	Construct Emergency Storage for Pacific PS	-	Criteria		12,000			70	840,000	294,000	1,134,000
P-9	Construct Emergency Storage for Voorhees PS	-	Criteria		10,500			70	735,000	257,250	992,250
P-10	Construct Emergency Storage for Meadows PS	-	Criteria		9,300			70	651,000	227,850	878,850
P-11	Construct Emergency Storage for Bell PS	-	Criteria		8,400			70	588,000	205,800	793,800
P-12	Construct Emergency Storage for Palm PS	-	Criteria		4,800			70	336,000	117,600	453,600
								Total	8,253,400	2,889,540	11,142,940

**Table 1-8
Wastewater Capital Improvement Program**

CIP No.	Project Description	Date of Construction for Existing Facility	Justification	Recommended Facilities				Unit Cost (\$)	Construction Cost (\$)	Eng. & Admin. Cost (\$)	Total Project Cost (\$)
				Firm Capacity (gpm)	Volume (gal)	Pipe Size (in)	Pipe Length (ft)				
1	Replace Poinsettia PS	1949	Condition / Criteria	150					2,000,000	700,000	2,700,000
	Replace Poinsettia PS Forcemain	1949	Age / Condition			4	163	300	49,000	18,000	67,000
2	Replace Pier PS Forcemain	1935	Age / Condition			4	900	400	360,000	126,000	486,000
3	Upgrade Pacific PS	1953	Criteria	400					400,000	140,000	540,000
	Replace Pacific PS Forcemain	1953	Age / Condition			6	1,225	240	294,000	102,900	396,900
4	Upgrade Voorhees PS	1953	Criteria	350					400,000	140,000	540,000
	Replace Voorhees PS Forcemain	1953	Age / Condition			6	930	240	223,200	78,120	301,320
5	Upgrade Meadows PS	1953	Criteria	310					400,000	140,000	540,000
	Replace Meadows PS Forcemain	1953	Age / Condition			6	730	240	175,200	61,320	236,520
6	Upgrade Bell Pump Station	1938	Criteria	300					400,000	140,000	540,000
	Replace Bell PS Forcemain	1938	Age / Condition			6	900	240	216,000	75,600	291,600
7	Replace Palm PS Forcemain		Age / Condition			4	775	240	186,000	65,100	251,100
8	Construct Emergency Storage for Pacific PS	-	Criteria		12,000			70	840,000	294,000	1,134,000
9	Construct Emergency Storage for Voorhees PS	-	Criteria		10,500			70	735,000	257,250	992,250
10	Construct Emergency Storage for Meadows PS	-	Criteria		9,300			70	651,000	227,850	878,850
11	Construct Emergency Storage for Bell PS	-	Criteria		8,400			70	588,000	205,800	793,800
12	Construct Emergency Storage for Palm PS	-	Criteria		4,800			70	336,000	117,600	453,600
*13	Gravity Sewer Rehabilitation and Replacement	Varies	Condition								28,930,000
*14	Manhole Rehabilitation and Replacement	Varies	Condition								596,000
**15	Collection System Capacity Deficiencies	Varies	Criteria			12 & 15	1,725				1,373,700
								Total	8,253,400	2,889,540	42,042,640
Notes: * Total project cost includes a projection for the remainder of the system to be CCTV inspected											
** Flow monitoring and verification of d/D ratios is recommended prior to project implementation											