#### **SECTION 1**

#### **EXECUTIVE SUMMARY**

#### 1-1 INTRODUCTION

#### **Background**

The City of Manhattan Beach is a full service City with a population of 36,718. The City provides sewer services to approximately 3.9 square miles of the land located within its corporate boundaries.

The City's existing wastewater collection system is made up of a network of gravity sewers, pump stations, and force mains. The gravity system consists of approximately 81.6 miles (430,784 ft) of pipe and 2,086 manholes and cleanouts. The system also includes eight pump stations and 5,114 feet of associated forcemains.

#### Previous Studies and Improvements

The City's last Wastewater System Master Plan was prepared in 1994 by Kennedy/Jenks Consultants. It analyzed the wastewater collection and pumping system to provide service through a planning period that extended to the year 2010. The City's year 1990 population was approximately 32,023.

The 1994 Wastewater System Master Plan included hydraulic analysis of all pipes over 8-inches in diameter. Of those analyzed, only two pipes were identified to be hydraulically inadequate. Video inspections were completed in the "Sand" and "Tree" sections of the City and the facilities were found to be in good condition. An ongoing rehabilitation program was recommended at an estimated annual cost of \$250,000.

The pumping system was found to require the most immediate attention. It was recommended that pumps and motors be replaced at six of the primary pump stations. Replacement of the natural gas engines with dedicated generator sets; replacement of dry well ventilation; rehabilitation of pump station wiring, main breakers, starters, and disconnect switches; and conversion of analog alarm SCADA system to digital was also recommended for each station. The total pump station improvement costs were estimated at \$707,500.

Five wastewater pump stations were rehabilitated in 1998. These included Pacific Avenue Pump Station, Palm Avenue Pump Station, Meadows Avenue Pump Station, Voorhees Avenue Pump Station, and Bell Avenue Pump Station. Pumps and motors were replaced at each station. Diesel fueled generator sets were installed in the dry wells or on site. Ventilation and SCADA systems were also upgraded.

#### **Objectives**

The objective of this Wastewater Master Plan is to evaluate the City's sewer collection system to provide a framework for undertaking the construction of new and replacement facilities for the service area in an efficient and cost effective manner. As a planning document, it is general in nature and is predicated upon the best information available at this time. It will aid the City in meeting some of the requirements of the Statewide General Waste Discharge Requirements issued by the California Regional Water Quality Control Board on May 2, 2006.

# 1-2 STUDY AREA

# Service Area

The City of Manhattan Beach (City) is located on the western edge of Los Angeles County, approximately 22 miles southwest of downtown Los Angeles. The City encompasses approximately 3.9 square miles of residential, commercial, and industrial land. It is a coastal community bounded by the approximately 2 miles of beach frontage to the west. Manhattan Beach is bordered by the City of El Segundo to the north, the Cities of Hawthorne and Redondo Beach to the east, and the Cities of Redondo Beach and Hermosa Beach to the south.

# **Topographical Description and Geology**

The characteristic topography of Manhattan Beach is a series of peaks and valleys throughout the City. The City's highest point is located near the intersection of Sepulveda Boulevard and Longfellow Drive and reaches 235 feet above sea level. The lowest ground elevation is at 0.0 ft, along the beach frontage.

The study area can be roughly divided into two sections based on its topography and soil conditions. A sand dune ridge runs diagonally from a point on the northwest City boundary approximately 2,000 feet from the coast to a point on the southern City boundary approximately 1,000 feet east of Sepulveda Boulevard. To the west of this ridge, the soil is fine dune sand and the topography is hilly. To the east of the ridge, the soil is sandy loam and the land is flat to gently sloping.

# <u>Climate</u>

The study area has a Mediterranean climate, enjoying plenty of sunshine throughout the year, with an average of 263 sunshine days and only 35 days with measurable precipitation annually. The period of April through November is warm to hot and dry with average high temperatures of 71 - 79°F and lows of 50 - 62°F. The average annual rainfall of about 13.19 inches occurs primarily during the winter months, between November and March.

# Land Use

The City is a well planned urban community with a balance of residential, commercial, and industrial land uses. The primary land use is residential (1,422 net acres or 70.5 %). The total study area includes approximately 2,017 net acres.

# Population

The City of Manhattan Beach had a population of 36,718 in 2009. With the total number of housing units at approximately 15,580 and a 3.72 percent vacancy rate, the population per household is estimated to be 2.45 (California Department of Finance, Demographic Research Unit).

# 1-3 CRITERIA

# <u>General</u>

Establishing performance standards is an important part of evaluating existing wastewater collection systems, as it forms the basis for system analysis and system improvement recommendations. These standards include methodology for estimating wastewater design flows and minimum design standards for the collection system pipes, pump stations, and forcemains.

### Flow Monitoring

A temporary flow monitoring study was conducted by ADS Environmental Services from December 21, 2008 to March 14, 2009 at eight locations. The flow monitoring sites were strategically selected to aid in the development of unit flow factors and calibration of the model. Sites were selected in an attempt to get a good sampling of data across the study area. At the same time, the areas tributary to each site must generate depths of flow large enough to develop accurate wastewater flows.

## Unit Flow Factors

Unit flow factors utilized in this study were developed based upon the existing land uses obtained from the City's GIS, and results of the flow monitoring studies. Water use records, aerial photographs and field reviews supplemented this information.

The average daily flow recorded at each flow monitoring site was utilized in determining calibrated existing unit flow factors for each land use. The flow factors were developed in units of gallons per day per acre.

## Peaking Factors

The adequacy of a sewage collection system must be based upon its ability to convey the peak flows. The temporary flow monitoring data was reviewed to develop peaking relationships at each site. As expected, these relationships varied from site to site depending upon the makeup and size of the tributary land use. Based upon the information developed, the following peaking relationships were selected for this study:

PDWF (mgd) = 2.35 x ADWF (mgd) <sup>0.92</sup> PWWF = 1.35 x PDWF Where, ADWF = Average Dry Weather Flow PDWF = Peak Dry Weather Flow PWWF = Peak Wet Weather Flow

#### Sewer Design Criteria

Design criteria are established to ensure that the collection system can operate effectively under all flow conditions. Each pipe segment must convey peak wet weather flows without surcharging the system. Low flows must be conveyed at a velocity that will prevent solids from settling and blocking the system. Details of the criteria recommended for the collection system and lift stations, and service to Specific Plan and Development sub-areas are included in Section 4 of this report.

# 1-4 EXISTING WASTEWATER COLLECTION SYSTEM

# General Description

The City of Manhattan Beach's existing sewer collection system, shown in Figure 1-1, is made up of a network of gravity sewers, pump stations, and force mains. The gravity system consists of approximately 81.6 miles (430,784 ft) of pipe and 2,086 manholes and cleanouts. The system also includes eight pump stations and 5,114 feet of associated forcemains.

The majority of the local sewers tie into one of the Los Angeles County Sanitation District (LACSD) trunk sewers crossing through the City. The sewage is then transported to LACSD's Joint Water Pollution Control Plant (JWPCP) in the City of Carson.

The sewers are primarily constructed of vitrified clay pipe with sizes ranging from 6-inches to 21-inches in diameter. Approximately 78 percent of the pipes are 8-inches in diameter. The majority of the City's sewer system was constructed between 1920 and 1960.

#### Los Angeles County Sanitation District (LACSD) Wastewater Collection System

Regional wastewater services are provided to the City and neighboring agencies by the Los Angeles County Sanitation District (LACSD). The City is a part of LACSD's South Bay Cities District. Regional trunk sewers collect the sewage generated in the service area and transport it to LACSD's Joint Water Pollution Control Plant (JWPCP) for treatment. The JWPCP is located in the southwest corner of City of Carson just east of I-110 freeway.

There are two primary LACSD trunk sewers within the City. The first trunk runs northwest to southeast and parallels The Strand along the beachfront. The second trunk runs west to east and is located in 26<sup>th</sup> Place, Bell Avenue, 25<sup>th</sup> Street and Marine Avenue. Pacific Avenue Pump Station, Palm Avenue Pump Station, Poinsettia Avenue Pump Station, Meadows Avenue Pump Station and Voorhees Avenue Pump Station are tributary to this trunk sewer.

#### 1-5 PUMP STATIONS

The City of Manhattan Beach owns and operates eight (8) wastewater pump stations. Table 1-1 provides general information on each facility. Detailed description and planning level evaluation of each pump station are included in Section 6.

#### 1-6 HYDRAULIC MODEL AND COLLECTION SYSTEM CAPACITY ANALYSIS

#### <u>General</u>

To perform a detailed evaluation of the capacity of the sewer collection system, it is convenient to create a mathematical model that is capable of simulating the operating characteristics of the system. The simulations for this study were performed on a Microsoft Windows based computer utilizing software designed for the analysis of sewer systems. The software selected for this study is InfoSewer. It is an ArcGIS-based computer program with the ability to perform steady state analyses of the flows in the sewer system. InfoSewer offers direct ArcGIS integration allowing GIS analysis and hydraulic modeling to exist in a single environment. The program also manages and maintains the database that stores the sewer analysis input and output results. Manning's Equation is used for depth of flow calculations in the gravity sewer pipes.

#### Geometric Model

As a part of this Master Plan project, the City's Wastewater GIS was developed. As-built plans were georeferenced and the wastewater facilities were then digitized. Data was collected from the as-built plans and input into the GIS. The developed Wastewater GIS served as the basis of the system geometric model. The hydraulic model includes the entire sewer system that is owned and operated by the City, from the uppermost reaches of the system to its confluence with a Los Angeles County Sanitation District (LACSD) trunk sewer or until the flow exits the City into an adjacent agency facility.

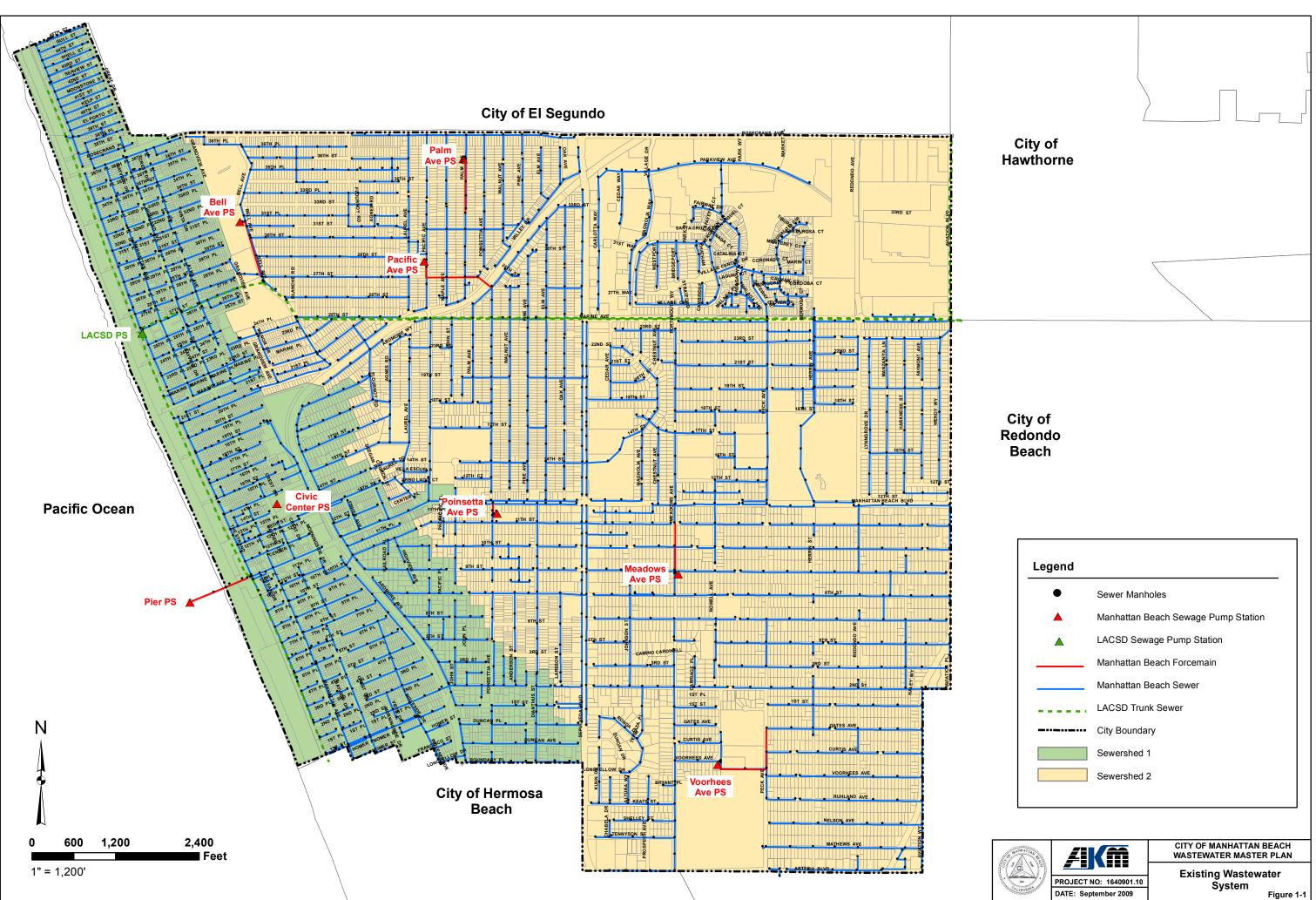


									Table 1 Pump Stat												
		Gen	eral Pur	np Station Ir	nformat	ion		-					Pump Specification	S	-	Motor	Specific	ations	Estir	mated F	lows
Pump Station	Location	Date of Cons.	Plan No.	Area Served (Acres)	Force Main Dia (in)	Force Main Length (ft)	Force Main Material	Туре	Wet Well Dimensions	Generator Set	Number of Pumps	Pump Mfg	Pump Type	Pump Model	Estimated Flow Capacity (gpm)	RPM	Phase	HP			PWWF (gpm)
Bell Avenue	Bell Ave between 31st St and 31st Pl	1938	S-60 S-150 S-230	78	6	900	CIP	Wetwell/ Drywell	12' x 9.5' x ~7'	Diesel Fueled	2	Flygt	submersible, non-clog	CT3102	263	1750	3	5	70	198	278
Palm Avenue	Palm Ave south of Rosecrans Ave	1953	S-149 S-230	39	6	775	CIP	Wetwell/ Drywell	10' x 4' x ~11.5'	Diesel Fueled	2	Flygt	submersible, non-clog	CT3127	174	1740	3	10	38	114	159
Pacific Avenue	Pacific Ave south of 31st St	1953	S-149	103	6	1,225	CIP	Wetwell/ Drywell	10' x 4' x ~11.5'	Diesel Fueled	2	Flygt	submersible, non-clog	CT3127	304	1740	3	10	98	269	376
Poinsettia Avenue	Poinsettia Ave south of Manhattan Beach Blvd	1949	S-227 S-145	25	4	163	CIP	Wetwell/ Drywell	18.6 ft <sup>2</sup> x 6'-10"	Diesel Fueled	2	Flygt	submersible, non-clog	CT3102	136	NA	3	NA	19	60	85
Meadows Avenue	Meadows Ave south of 9th St	1953	S-148 S-230	99	6	730	CIP	Wetwell/ Drywell	10' x 4' x ~11.5'	Diesel Fueled	2	Flygt	submersible, non-clog	CT3127	304	1740	3	10	77	216	303
Voorhees Avenue	Voorhees Ave at Rowell Ave	1953	S-148 S-230	103	6	1,321	CIP	Wetwell/ Drywell	10' x 4' x ~11.5'	Diesel Fueled	2	Flygt	submersible, non-clog	CT3127	227	1740	3	10	90	250	350
Civic Center	Highland Ave south of 15th St	1992	-	Civic Center Bulding	-	-	-	Submersible	-	-	2	-	-	-	-	-	-	-	-	-	-
Pier	End of Manhattan Beach Pier	1992	-	Pier Restaurant	4	-	CIP	Submersible	4' x 4' x 4'	-	2	-	submersible	-	-	-	-	3	-	-	-

# Land Use

The parcel layer, which provides the City's 2003 General Plan land use information, was used as the land use base map. Since the City's service area is mostly developed, the hydraulic analyses were conducted assuming fully developed and occupied tributary areas.

## Tributary Areas

Polygons were created around individual sewer manholes to define the tributary area to each manhole. Most manholes have a tributary area assigned to it unless there are multiple manholes in the same area. Approximately 1,530 polygons were created for the existing and ultimate system analysis.

# Model Loads

For each tributary area, the existing land uses and the developed unit flow factors were utilized to apply the average loads to the hydraulic model.

Peak dry weather flows are calculated in the model by a user defined relationship. The peaking formula used in the City's sewer model is as follows:

PDWF (cfs) = 
$$2.35 \times ADWF(cfs)^{0.92}$$

# Pumped Flows

There are two separate analysis methods that can be used on the City's sewer collection system to evaluate the effect of pumped flows downstream of the existing 8 pump stations. A description of each of the analysis methods is as follows:

Method 1: The average tributary flow to each pump station was transferred to the manholes located at the discharge end of the respective forcemains. The total average flow was peaked and the downstream sewer depth to diameter ratios were evaluated based on the calculated peak dry weather flows or

$$Qpdw = (2.35 \times Qadw^{0.92})$$
 (in mgd)

Method 2: The pump capacity of each pump station was transferred to the manholes located at the discharge end of the respective forcemains. The pump capacities were not peaked, but added to the peak dry weather flow at each manhole located downstream of the pump station. The downstream sewer depth to diameter ratios were evaluated based on the calculated peak dry weather flows plus the pump capacities (Qpump) or

$$Qpdw = (2.35 \times Qadwf^{0.92}) + Qpump (in mgd)$$

Flow monitoring results have shown that pump discharge becomes more and more attenuated the further downstream in the system the monitor is placed. Often times, only the first few reaches located immediately downstream of the discharge point will experience a rush of flow that reflects the pump capacity. In this study, the pump capacity was considered if flow monitoring results showed an influence from the pump station discharge. For the pipes located a far distance from the pump discharge point, the calculated peak dry weather flow based on land use and unit flow factors was utilized for analysis.

### Hydraulic Analysis Results

The depth to diameter ratio exceeded the established criteria of 0.64 at the following locations:

 Bell Avenue at 25<sup>th</sup> Street (Pipe ID 15084-070L4) 10" diameter sewer with peak d/D>1.00 No pump station influence

As-built plans show a slope of 0.00 and a length of 10 feet. The downstream manhole 070L4 is a LACSD manhole. It is recommended that the slope of this sewer be verified.

Pacific Avenue north of 27<sup>th</sup> Street (Pipe ID 08077-08082)
 8" diameter sewer with peak d/D=0.66 with no pump station influence
 8" diameter sewer with peak d/D>1.00 with pump station influence from Palm Avenue PS (see Table 7-2 and description below)

A few reaches downstream of the Poinsettia Avenue Pump Station, Palm Avenue Pump Station, and Bell Avenue Pump Station discharge locations resulted in d/D ratios greater than 0.64 when the pump capacities were implemented (Method 2). These locations are listed in Table 1-2. It is recommended that flow monitoring be conducted in these areas to verify the d/D ratios prior to implementing any replacement projects.

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	U/S MH	D/S MH	Dia	Length		ADWF	Pumped Flow	PDWF	PDWF Vel	PDWF	PDWF Depth	Full Flow	
Pipe ID	ID	ID	(in)	(ft)	Slope	(mgd)	(mgd)	(mgd)	(ft/s)	d/D	(ft)	(mgd)	Comments
06173-06174	06-173	06-174	8	192	0.0040	0.0019	0.7363	0.7437	3.30	1.00	0.67	0.4959	D/S of Poinsettia PS
06174-06175	06-174	06-175	8	254	0.0040	0.0028	0.7363	0.7469	3.31	1.00	0.67	0.4962	D/S of Poinsettia PS
06175-06176	06-175	06-176	8	190	0.0136	0.0033	0.7363	0.7485	4.51	0.69	0.46	0.9125	D/S of Poinsettia PS
08037-08052	08-037	08-052	8	262	0.0040	0.0446	0.4085	0.5430	2.41	1.00	0.67	0.4957	D/S of Palm PS
08052-08055	08-052	08-055	8	281	0.0040	0.0583	0.4085	0.5805	2.57	1.00	0.67	0.4944	D/S of Palm PS
08055-08077	08-055	08-077	8	350	0.0040	0.0637	0.4085	0.5951	2.64	1.00	0.67	0.4953	D/S of Palm PS
08077-08082	08-077	08-082	8	110	0.0040	0.0858	0.4085	0.6538	2.90	1.00	0.67	0.4951	D/S of Palm PS
16019-16022	16-019	16-022	8	68	0.0099	0.0131	0.5728	0.6164	3.84	0.67	0.45	0.7809	D/S of Bell PS
16022-070L3	16-022	07-0L3	10	18	0.0028	0.0135	0.5728	0.6176	2.38	0.69	0.58	0.7504	D/S of Bell PS
		1	「otal	1,725									

 Table 1-2

 Pipes with Model Calculated Capacity Deficiencies Downstream of Pump Discharge Points

# 1-7 COLLECTION SYSTEM CONDITION ASSESSMENT

#### <u>General</u>

Condition of the collection system depends on many factors, including construction quality, and maintenance practices, surrounding soil and groundwater conditions, flow quantity and velocity, and constituents in sewage. The best available information on the system condition is provided by the closed circuit television (CCTV) inspection reports and recordings and the knowledge of the maintenance staff.

The City of Manhattan Beach (City) established a program to begin CCTV inspections of its approximately 86 mile long sewer collection system. The scope of the Master Plan included inspections of 227,714 feet (43.1 miles) of pipe or 52.9 percent of the total system.

#### **Closed Circuit Television (CCTV) Inspections**

Empire Pipe Cleaning and Equipment, Inc. (Empire) performed video inspection work on approximately 222,714 feet of pipe between October 2008 and August 2009. National Association of Sewer Service Companies (NASSCO) Pipeline Assessment and Certification Program (PACP) coding procedures formed the basis of the inspection work and reports. Each inspection report lists the service connections and deficiencies by location in the inspected pipe. Photographs of the identified deficiencies are included in the inspection reports.

All data was compiled into one summary database, which can be found in Appendix 5. The database includes a summary count of the deficiencies identified in the CCTV written reports for the 1,200 reaches were inspected.

#### **Review of Representative CCTV Recordings**

The Inspection Report Database Summary was used in selecting the recordings to be reviewed in detail. The pipe reaches selected for detailed review were those that showed the most severe structural problems and multiple deficiencies, such as deformed pipe, hole in pipe, broken pipe, large offset joint, large obstacles, multiple deficiencies such as fractures, cracks, roots, deposits, obstructions, sags, camera underwater, and survey abandoned, and ball roots, as well as severe operation and maintenance issues. Several reaches without listed deficiencies were selected in order to develop insight into the overall condition of the CCTV inspected system. Recordings for 264 reaches and 53,789 feet of pipe (23.9% of total inspected) were selected and reviewed in detail.

#### **Condition Grading**

The PACP condition grading system was used to assign a condition rating for structural defects and operation and maintenance defects for each reach of pipe. The rating provides the ability to quantitatively measure the difference in pipe condition between one inspection and subsequent inspections, and to prioritize among different pipe segments. A grade of 1 to 5 is assigned to each defect based on potential for further deterioration or pipe failure. Pipe failure is defined as when it can no longer convey the design capacity. The grades are as follows:

- 5 Immediate Attention Defects requiring immediate attention
- 4 Poor Severe defects that will become Grade 5 defects within the foreseeable future
- 3 Fair Moderate defects that will continue to deteriorate
- 2 Good Defects that have not begun to deteriorate
- 1 Excellent Minor defects

The problems identified most often were fine roots (835 reaches, 22% of total), cracks (563 reaches, 15% of total) and joint offset medium (437 reaches, 12% of total).

### Replacement and Rehabilitation Priorities

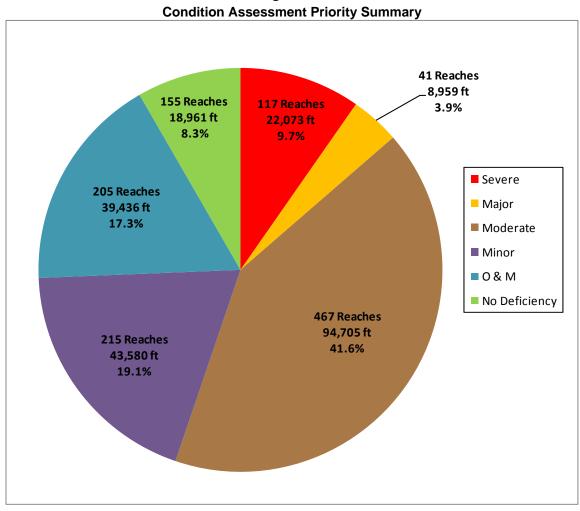
The purpose of CCTV inspections is to determine the condition of the City's existing gravity sewers, and formulate a rehabilitation plan for the defective sewers. The defect scores and indexes provide a good indication as to which pipes are in poor condition, but cannot be relied upon solely to prioritize improvement projects. The priorities are selected primarily with consideration of the health and safety of the public and protection of the environment by minimizing the possibility of sanitary sewer overflows and leakage. The pipe capacity, location of particular defects, and the tributary areas/wastewater flow rates are other considerations used in formulating the final capital improvement project priorities.

The initial priorities for improvements to the sewers are based on the severity of the pipe defects. The six (6) categories utilized in this report are as follows:

- a. <u>Severe Condition</u> This category primarily includes structural defects of deformed pipe, hole in pipe, broken pipe, and large joint offsets.
- b. <u>Major Condition</u> This category primarily includes structural defects of multiple fractures, medium joint offsets and major sags. Pipes with a large number of cracks are also included.
- c. <u>Moderate Condition</u> Pipes in this category have fractures, cracks, small and medium joint offsets, and sags.
- d. <u>Minor Condition</u> Pipes in this category have slight sags, cracks, and small joint offsets.
- e. <u>O&M</u> This condition is for operational and maintenance problems and construction feature defects. There are no structural defects.
- f. <u>No Defects</u> This condition is for the pipe with no structural, operation and maintenance or construction feature defects.

Figure 1-2 shows the distribution of the condition priorities assigned the pipes with completed inspections.

The City of Manhattan Beach will address the "Severe" and "Major" collection system deficiencies. The collection system construction estimates are based upon replacement at \$45 per diameter inch per foot of pipe. Implementation cost is determined by adding 35 percent of construction cost to cover engineering, inspection, and administration. The total estimated cost of upgrading the sewers with severe or major condition priorities is \$15,330,000. This is based on the assessment of 43.1 miles of pipe or 52.9 percent of the total gravity sewer system. Assuming that similar deficiencies will be identified when the remainder of the system is CCTV inspected, the total cost for rehabilitation and replacement of the remaining gravity sewer pipes is estimated at approximately \$13,600,000.



# Figure 1-2

#### Follow-up CCTV Inspection and Condition Assessment Program

- a. Portions of the system rated to be in Severe Structural Deficiency condition will be inspected annually and evaluated to determine if immediate corrective action is needed.
- b. Portions of the system rated to be in Major Structural Deficiency condition will be CCTV inspected and evaluated once every three (3) years
- Portions of the system rated to be in Moderate Structural Deficiency condition will be CCTV c. inspected and evaluated once every five (5) years
- d. Portions of the system rated to be in Minor Structural Deficiency condition will be CCTV inspected and evaluated once every ten (10) years
- e. Portions of the system with no structural deficiencies will be CCTV inspected and evaluated once every ten (10) years

- f. Portions of the system with **Operational and Maintenance** deficiencies, except the **Hot Spots**, will be CCTV inspected and evaluated once every **four (4) years.**
- g. **Hot Spots**, except siphons, will be CCTV inspected and evaluated **before and after each maintenance activity and cleaning for one year** to establish the appropriateness of the method, and then **annually**.

As structural deficiency mitigation projects are implemented, their condition will be reclassified, and they will be included in the appropriate category for follow up CCTV inspection and condition assessment work.

#### Manhole Inspections and Assessments

The condition of the manholes associated with the 52.9 miles of gravity pipes inspected was also assessed. A total of 1,075 manholes were inspected and assessed. Each element was rated as good, fair, poor, or failing. Signs of debris, grease, vermin, and odors were also noted in the inspection reports. The results of the inspections are summarized in Table 1-3.

	M	anhol	e Eler	nents	Conc	lition	Sumn	nary			
Condition	Manhole Cover	Frame	Cone	Barrel / Wall	Rungs	Bench	Channel	Debris	Grease	Vermin	Odor
Good	1070	1068	1030	1048	612	1022	960				
Fair		7	37	22	155	38	95				
Poor			7	4	150	15	20				
Failing			1	1	67						
Broken	2										
Cracked	3										
Not Applicable					91						
Yes								15		55	3
No								1060	1075	1020	1072

 Table 1-3

 hhole Elements Condition Summar

The total estimated cost of upgrading the manholes with poor conditions is \$316,000. This is based on the assessment 1,075 manholes or 53 percent of all the manholes. The total number of manholes in the sewer system is estimated at 2,031. Assuming that similar deficiencies will be identified when the remainder of the system is CCTV inspected, the total remaining cost for rehabilitation and replacement of the manholes in poor condition is estimated at approximately \$280,000.

#### 1-8 OPERATION AND MAINTENANCE

#### **CCTV Inspection Data**

The operation and maintenance conditions identified through CCTV inspections include ball roots, grease, sags, deposits, obstacles, and infiltration, respectively. These conditions are the cause of many of the

frequent maintenance areas (hot spots). Reduction in cross sectional pipe area is a potential cause for back ups and sewer overflows. Accordingly, the highest priority is given to those reaches identified with ball roots.

The recommendations provided should be used in updating the operation and maintenance of the system, the Hot Spot maintenance program, the cleaning program, as well as the Preventative Maintenance Program.

#### <u>'Hot Spots'</u>

Hot Spots are areas of the system with reoccurring problems that require maintenance and cleaning on a quarterly basis minimum. Currently, there are 76 reaches with a total length of 10,796 feet that are considered to be Hot Spots in the City's system. Operations staff reports that the causes of the hot spots are grease and roots.

#### Sanitary Sewer Overflow (SSO) History

There were a total of 19 sanitary sewer overflows responded to by City crews between January 2007 and June 2009. The total number of reported spills that occurred on City property over the past three years is as follows:

7 spills in 2007 (8.6 spills per 100 miles)
3 spills in 2008 (3.7 spills per 100)
1 spill in 2009 (1.2 spills per 100) - up to September 2009

A sewer collection system with less than three (3) spills from the publicly owned system (excludes private property spills that do not result from a blockage in the public system) per 100 miles per year is considered an adequate system. For the City's sewer system (81.6 miles), this is an average of about 2.4 (3 x 0.816) spills per year.

#### Maintenance Program

A comprehensive maintenance program is an important tool in assuring reliable system operation. This not only includes regular inspections and preventative maintenance, but also good record keeping. Accurate records are the backbone of any maintenance operation. They can be used for many purposes including: scheduling regular maintenance activities; allocating manpower; budgeting; pinpointing persistent problems; tracking equipment performance and maintenance history; and the identification of equipment which may be showing signs of failure.

#### Preventative Maintenance

Preventative maintenance is a crucial element of the maintenance program. The preventative maintenance program (PMP) consists of cleaning, inspection, condition assessment, and rehabilitation tasks.

Sewer inspection includes CCTV inspection and condition assessment of the collection system, visual inspection of manholes and their flow channels, ground surface inspection of rights of way and easements, and odor and corrosion monitoring. Condition assessment includes, review of the inspection data, and formulation of maintenance, rehabilitation, and replacement projects. Following the completion of the initial CCTV inspection program, the City should develop a continuing inspection plan based upon the knowledge

gained from the initial program. Hot Spots, except siphons, should be CCTV inspected and evaluated before and after each maintenance activity and cleaning for one year to establish the appropriateness of the method, and then annually. Each spill site must be CCTV inspected to pinpoint the cause of the spill, and implementation of corrective measures for preventing repeat spills.

Preventative maintenance activities that the City does currently conduct include the following:

- > Clean entire gravity system twice per year
- Clean hot spots twice per month
- Inspect and skim wet wells twice per week
- Vacuum out wet wells once per year
- > Inspect pump stations once per week by Utilities Electrician and Sewer Maintenance Worker
- Diagnose pump station motor control centers annually for operational efficiency and safety by the Southern California Edison Company
- > Catalogue and track tasks by work orders in a maintenance management system
- > Perform routine mechanical root removal in areas of known root intrusion
- Start test the emergency power systems at the pump stations weekly
- Service generators bi-annually by Power Plus, Inc.

#### Maintenance Staff and Training

The City currently has about 81.6 miles of pipe. In order to comply with the WDR requirements and the City's regular preventative maintenance program, the City must quantify the number of employees and equipment necessary to perform these tasks.

The City's current staffing for the wastewater collections system maintenance includes 5 employees, one supervisor and four field staff members. Training of these staff members is as follows:

- a. Initial training in proper operation and maintenance of all major wastewater equipment and facilities by the Wastewater Supervisor
- b. Critical equipment training is facilitated by outside contractors in areas of fork lift operation, backhoe and bob-cat operation, and crane operation
- c. Periodic training in confined space entry, trench shoring, traffic control, heat illness, and hazardous materials management
- d. Task proficiency is required for all positions and training records are maintained to monitor completed classes
- e. Disaster training exercises are conducted twice per year
- f. Staff is also trained in Best Management Practices of the Fats, Oils, and Grease (FOG) Control Ordinance and is instructed to report violations

## 1-9 CAPITAL IMPROVEMENT PROGRAM

### **General Description**

The primary goal of the Capital Improvement Program (CIP) is to provide the City of Manhattan Beach with a long-range planning tool for implementing its sewer infrastructure improvements in an orderly manner and a basis for financing of these improvements. To accomplish this goal, the program is phased based upon the implementation cost of the facilities, the quantity of work the City can reasonably administer each year, and the funds available for these projects.

## Capital Improvement Project Priorities

The capital improvement projects were selected primarily with consideration of the health and safety of the public and protection of the environment by minimizing the possibility of overflows. The projects that will eliminate the capacity deficiencies in the gravity collection system are prioritized based upon the hydraulic analyses conducted during this study. As the City completes CCTV inspection of the system, severe and major defects identified should be incorporated into the CIP and addressed. When the CCTV inspection is completed and a full condition assessment has been conducted, the capital improvement project priorities should be reevaluated.

## Collection System Capacity Improvement Projects

The collection system capacity improvement projects include the areas identified with a capacity deficiency in the hydraulic model when pump capacities were implemented. It is recommended that the identified locations be flow monitored to verify the d/D ratios prior to implementing any replacement projects. Operations staff has not indicated that these areas are a problem. Therefore, until the deficiencies are verified in the field, these projects are considered low in priority.

#### Collection System Condition Improvement Projects

The condition improvement projects are prioritized solely on the condition of the pipe as determined from reviews of the CCTV recordings. The condition deficiencies with critical structural damage and severe obstructions were given the highest priority. Sewer pipes with conditions categorized as "Severe" or "Major" and manholes categorized as in poor condition are included in the recommended improvements.

The planning level recommendations are based upon the ranking and pipe defects from the CCTV inspection reports, and reviews of recordings. It may be possible to reline, repair or perform root treatment on some of the existing gravity pipes, in lieu of replacing them. Actual improvements should be designed based upon further detailed reviews of each recording, taking into consideration other factors such as location, age, capacity of the pipe, existing utilities, and concurrent infrastructure construction projects.

The useful life gained from replacing the deficient facilities will be longer than repairs and relining projects. Root treatment is usually a temporary solution. Unless the source is removed, it is likely that the roots will get thicker as time passes and the root intrusion will continue until the pipe is replaced.

#### Pump Station Improvements

The recommended pump station capital improvement projects have been based upon condition assessment of each facility, capacity analysis, and conformance with the adopted criteria. The implementation priorities should be based upon the likelihood of a failure that may result in a spill, the volume of spill, and its impact on the public and the environment. The condition assessment and analysis results are described in detail Section 6 for each pump station.

#### **Capital Improvement Program**

The Capital Improvement Program is developed based upon the results of the hydraulic analyses and the established priorities. The recommended improvement project locations are illustrated on Figure 1-3.

Gravity collection system projects are listed in Table 1-4 and Table 1-5 by priority, along with cost estimates. The cost estimates presented in Table 1-4 and Table 1-5 reflect replacement of the existing facilities. Replacement costs are generally more conservative and will therefore allow the City more flexibility for each project. The pipeline construction costs are based upon \$45 / diameter inch / ft. Preliminary design studies should be conducted utilizing detailed utility information to identify and evaluate project alternatives such as parallel pipes and/or diversions prior to final design. When sewers are replaced, they should be relocated into the walk ways or right-of-ways as much as possible if they are currently in a location that cannot be easily accessed.

The City of Manhattan Beach is largely occupied and there are many existing utilities to consider. Therefore, the costs of replacing sewer facilities will be generally higher than in an area that is undeveloped. The total costs shown in Table 1-4 and Table 1-5 include engineering, administration and contingency costs, estimated at 35% of the construction cost.

The manhole rehabilitation and replacement projects are listed in Table 1-6. The pump station and forcemain improvement projects are listed in Table 1-7.

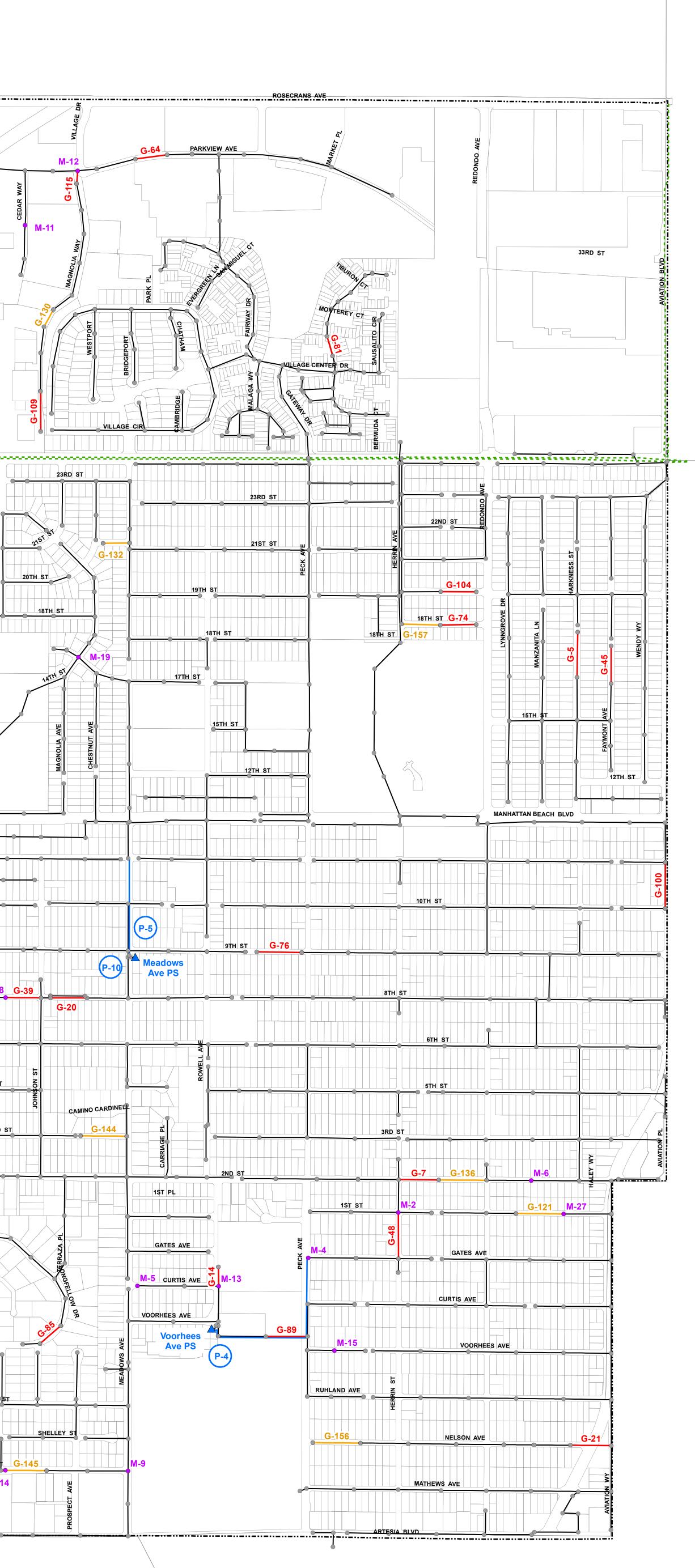
The recommended projects have been based upon the best information currently available. It should be updated as new information becomes available from sources such as CCTV inspections and from maintenance crew observations. The project priorities may be revised to correspond to changed conditions, such as impending facility failures, or to take advantage of concurrent construction such as street paving projects or adjacent infrastructure work.

Some of the projects recommended are small and it may not be feasible to implement them as a single project. Therefore, several projects should be combined and bid as a package. Some of the projects may be broken down into smaller components to fit the City's budgetary and other obligations.

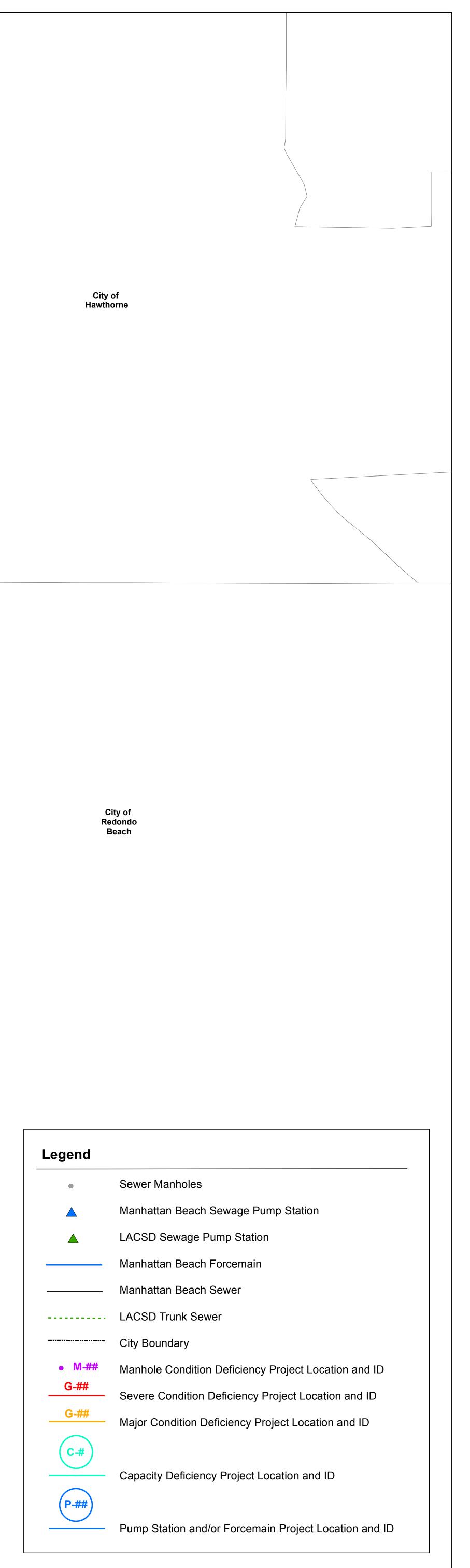
The total Wastewater Capital Improvement Program is shown in Table 1-8. The total CIP costs are estimated at \$42,042,640.













**CITY OF MANHATTAN BEACH** WASTEWATER MASTER PLAN **Capital Improvement** 

Projects Figure 1-3

				-	Oonee		Stenre	Jupacity	improv	ement Proj		-			
Project No.	Pipe ID	U/S MH ID	D/S MH ID	Existing Dia (in)	Length (ft)	Slope	ADWF (mgd)	Pumped Flow (mgd)	PDWF (mgd)	Comments	Replacement Size (in)	Unit Cost (\$)	Construction Cost (\$)		Project Cost (\$)
	06173-06174	06-173	06-174	8	192	0.0040	0.0019	0.7363	0.7437	D/S of					
C-1	06174-06175	06-174	06-175	8	254	0.0040	0.0028	0.7363	0.7469	Poinsettia	15	675	429,300	150,255	579,555
	06175-06176	06-175	06-176	8	190	0.0136	0.0033	0.7363	0.7485	PS					
	08037-08052	08-037	08-052	8	262	0.0040	0.0446	0.4085	0.5430						
C-2	08052-08055	08-052	08-055	8	281	0.0040	0.0583	0.4085	0.5805	D/S of Palm	12	540	5/1 659	189,580	731,238
0-2	08055-08077	08-055	08-077	8	350	0.0040	0.0637	0.4085	0.5951	PS	12	540	541,050	109,000	131,230
	08077-08082	08-077	08-082	8	110	0.0040	0.0858	0.4085	0.6538						
C-3	16019-16022	16-019	16-022	8	68	0.0099	0.0131	0.5728	0.6164	D/S of Bell	12	540	46,591	16,307	62,898
0-5	16022-070L3	16-022	07-0L3	10	18	0.0028	0.0135	0.5728	0.6176	PS	12	540	40,391	10,307	02,090
				Total	1,725									Total	1,373,691

 Table 1-4

 Collection System Capacity Improvement Projects

																		Structura	Defeed	0		ravity	Sewer	Rehabl	lable 1-		laceme	ent Pro		tional an	d Melata							Constructi	on Eestur	-				ाचा			
Project No.	DVD No.	Inspection No.	Reversai DVD No.	Reversal Inspec. No. DVD Watched? (Y)	Location Street Name	CCTV Date			nhole End	Direction of Carmera	in) j Mat	1 (ft) 1	(11)		F		V SV V	niot	t S V VIL	Deformed	<ul> <li>V Collapsed Pipe</li> <li>Surface Damage</li> </ul>		Ø   ⊢		Deposi D AGS AE ( 2 2	Other B	LJC	Tap BL	Roots (R (T) M J C B	i) Iedium 1. J. C	Bell (B B L J	5) [C G [ D	RW	Obstaci es OB Other C C Z %	C R B	Total O&M Defect Score	Tap (Laten T FD BI	al) BD D]L]1	Line	Intrud Ing Seal Mate iai	3		Condition Ranking	No Reverse Set-up Complete	Commente	Minimum Recommendations	Project Replacement Cost (\$)
G-1	G312-011 Ph 2	1 11	G312-011 Ph 2		1ST ST	7/21/09	12025A- 12025	12-025A	12-025	D/S 1			80			1		1			1		3	3 13 4.			7				1					B 12	2				2	Seve	ere 1	E f	64' Collapsed Pipe & 64.8' BPSV. MSA 64.2' (16.2' from D/S MH) BPVV. Inspection complete.	Spot repair	58,466
G-2	G312-003 Ph 2			Y	THE STRAND	7/9/09	14070- 14071	14-070	14-071	u/s	6 VCP	181	5				1					1	2	2 6 3.	0									1 30		1 4 4.	.0				1	Seve	ere 2	: × (	3' BPVV & 5' BPSV. 5' MSA (OBZ=OBI, Another Utility Pipe). Camera blocked by obstacle thru wall. RP=RPP.	Replace pipe	65,975
G-3	G312-004 Ph 2				MANHATTAN BEACH BLVD	7/10/09	14059- 14060	14-059	14-060	u/s	6 VCP	70	144			3 1	1	3					s	34 3.	8 1	1	1							1 30		4 92	.3					Sev	ere 3	)           	55.4' BPSV (Another sewer line or sewer lateral in the main sewer pipe), & 108.4' BPVV. inspection report shows Repair Patch at 104.8'. But looks like BPVV. OBZ-OBI	Spot repair or Replace pipe	52,634
G-4	G312-056	6 8		Y	26TH PL	1/13/09	20008- 20009	20-008	20-009	D/S	8 VCP	152	161		2		2	2		1			7	27 3.	9		1 18 1		2	1 3					2	6 40 1	.5					Sev	ere 4	1	144.2' to 146.6' & 157.1' BPVV. 144.2' to 146.7' Deformed Pipe Horizontal.	Spot repair 144' to 148 'BPVV & Deformed Pipe. Also Spot repair 157.1' BPVV. Root Treat and Cut.	78,149
G-5	G312-006	64	G312-006	6 Y I	HARKNESS ST	10/27/08	02006- 02007	02-006	02-007	u/s	8 VCP	323	326	1		2	3			з			S	<b>4</b> 0 4.	4	1	28	1	1	1	1				1	5 24 1	.6 1				2	Sev	ere 5	5	306.7' to 316' Deformed Pipe (Also BPVV & Continuous Fracture Multiple). MSA 311.1' (14.8' from U/S MH) BPVV.	Replace 303' to 316' to Deformed Pipe	158,387
G-6	G312-033	35	G312-033	6 Y 9	9TH ST	12/5/08	11093- 11094	11-093	11-094	D/S	6 VCP	271	278	19		3 1		20		1	11		55	5 161 2.	9		12		1	1				1 15	1	5 21 1	.4				2	Sev	ere 6	5	55' to 61.5' Deformed Pipe Vertical & BPSV 57.8' from DS MH. MSA 137.8' (140.3' from D/S MH) Obstacie. SAVZ. OBJ	Spot repair 55' 10 61' Deformed & Broken Pipe (SV). Clean obstacle @ 137' from U/S MH	101,367
G-7	G312-002	2 7		Y	2ND ST	10/21/08	01082- 01083	01-082	01-083	D/S	8 VCP	296	303	2 2	7 2 1 1	10	3	3		2			4 36	5 117 3.	3 100		2								10	2 202 2	.0	1				Sev	ere 7	,	124.5' & 142.8' Deformed Pipe Vertical. 124.5', 142.8' & 150.8' BPVV. 120' to153.9' Continuous Fractures.	Deformed pipe, BPVV, & fractures.	147,404
G-8	G312-008 Ph 2		,	Y	8TH ST	7/16/09	11099- 11100	11-099	11-100	D/S	8 VCP	190	164	2 2 1	2	5	2			1	5		30	88 2.	9 1		1 8		1	1	2				1	4 24 1	.7 2	2				Sev	ere 8		99' BPVV. 156.3' BPVV & Deformed Horizontal Pipe. S=SAVZ	99' Spot repair & Replace pipe 150' to159'	79,461
G-9	G312-008 Ph 2				8TH ST		11100- 11101		11-101	D/S	8 VCP	P 120	131	1			2			1 1	6		21	65 3.	.1		2 8	1	1						1:	2 15 1	.3					Sev	ere 9		124.9 Deformed Pipe Horizontal & BPVV. 126.8' Deformed Pipe Vertical & BPVV. S=SAVZ	Replace pipe 124.9' to 127'	63,423
G-10	G312-021	1 3		Y	ROSECRANS AVE	11/17/08	07001- 07002	07-001	07-002	u/s	8 VCP	198	185			2				1				3 13 4.	3		2				<b>       </b>					2 2 1	.0	1				Sev	rere 10	<b>°</b>   1	148' to 151' Deformed Pipe Horizontal. 15' to 15.8' Deformed Pipe	Spot repair 148' to 151' to Deformed H. Pipe	90,007
G-11	G312-035	5 1		Y	3RD ST	12/9/08	12031- 12032	12-031	12-032	u/s	6 VCP	138	16	1	1 2		1			2	1		3	3 26 3	3		1		_	1						2 42	.0				,1	Sev	rere 11	1 x	Horizontal & BPVV. 15.8' MSA (BPVV). U/S MH is a possible C/O. SAVZ	Spot repair or Replace pipe 8' 1o 17'	50,122
G-12	G312-036	66		Y	18TH ST	12/10/08	15036		15-036	D/S	8 VCP	302	307		1		1	11		1			6	5 20 4	0		2 12								1	7 22 1	.3					Sev	rere 12	2	304.0 JOL (D/SH).	Spot repair 301' to 305' Deformed Horizontal & BPVV	
G-13	G312-060	0 1		Y	MARINE AVE		21012- 21016	-	21-016	D/S	6 VCP	° 303	310	4	9 1	3	1	83					29	9 89 3	.1	$\square$	48				2				5	0 56 1	.1 1					Sev	vere 13	3	JOL (D/SH) & 150.4 JOL (D/SL). 175.1' BPVV. 120.9' JOL (D/SL). 123'	Spot repair 139.1' 1o 150.4', & 175.1'	112,959
G-14	G312-013	3 5	G312-013	3 Y	ROWELL AVE	11/5/08	B 05054- 05055	05-054	05-055	D/S	6 VCP	P 144	151					3						3 15 5	.0											0 00	.0				2	Sev	vere 14	4	JOL (D/SH) & 148.9' JOL (D/SL). MSA 121.1' (JOL)	Spot repair 120.9', 123' & 148.9' to JOL	55,003
G-15	G312-072	2 8	G312-060	2 Y	20TH ST	2/11/09	22030- 22031	22-030	22-031	D/S	6 VCP	342	357	1 1 1	3 2	2	2	10 2			41		74	4 186 2	.5 1		5 24	4		3 1 1	6				4	5 78 1	.7				2 1	Sev	vere 15	5	6.5' &134.6' BPVV. 48' JOL (D/SL). 171.3' from D/S MH MSA (JOL). Surface Damage= SSSZ 205.5' & 208' JOL (D/SH).	Spot repair. Roof Treat & Cut	130,090
G-16	G312-057	7 10	G312-058	1 Y	VISTA DR	1/14/09	21002- 21005	21-002	21-005	u/s	6 VCF	256	247	1	1	1	1	9 2					15	5 51 3	.4		2 13			2 1	5				2	3 44 1	.9				2	Sev	vere 16	6	208' JOL could be size change, not clear. 101.2' BPVV. MSA 240' (6.5' from U/S MH) RMJ.	Spot repair 101.2' BPVV& & 205.8 to 208' for JOL. Root Treat & Cut	89,849
	G312-017 -	_			3RD ST		9 10139- 10142	10-139	10-142						5 1	2		2 2 1				2	2 22			1	1 5 15			2						3 28 1			1		1			7 x	165.6' MSA (JOL). 133.3' & 133.8' HVV. 165.1' JOL (D/SH). After 165.1' there is another JOL or Joint Angular. RP=RPR 129.2' BPVV & Deformed	Spot repair or Replace pipe	60,361
	G312-017 - G312-005				6TH ST	8/3/09	9 11120A 9 11122 17023-	11-120A	11-122	++		+	131	2 2	Б		1		++		4		12 17	+ $+$	+ $+$ $+$	-+	1 19	$\left  \right  \right $		31		┟╂╂	$\left  \right $	┼┼┼	2	4 35 1 3 3 1			+		++		vere 18		Pipe Horizontal. S=SSSZ 4', 74.1' & 75.9' JOL (D/SH).	Replace pipe after 129'	47,859 54,626
G-19 G-20	Ph 3	°	<b>├</b> ──-		EASEMENT 8TH ST		9 17023- 17024 04059-		17-024			-	112 278	+++	╫╫		+	3		+			13 18		+ $+$ $+$	-+		┼┼┼		$\left  \right $	┼┼┼	┼┼┼	┼╀╶┧	┼┼┼	┼┼	3 31 0 00	.0		┼╫╌┥		++		vere 19		248.6' JOL (D/SL). 250.6'	Spot repair 248' to 251' JOL	202,370
		+	00/0			1	04060	04-059				+		+		++	╉╋	2		┼┼						+		┼┼┼			┼┼┼	╉╂╂	╁┼┤	┼┼┼			7		┼┼┤						JOL (D/SH). 225.9' JOL (D/SL). 238.9' JOL (D/SH). MSA 239.5'	(D/SL) & JOL (D/SH) Spot repair JOL (D/SL) 225.9	+
G-21 G-22	G312-003 G312-031		G312-003 G312-031	3 V	NELSON AVE	10/22/08	01032	11.040	01-032	┼╌┼╴	8 VCF 6 VCF	+		3	4 1			10 2					20	1 34 3 0 60 3			9	1 1		1					3	5 61 1 3 19 1	.5				2	$\left  \right $	vere 21		(67.2' from D/S MH) JOL. 67' JOL (D/SL) & 68.5' JOL	& JOL (D/SH) 238.9' Spot repair 67' to 68.5 to JOL	149,056
							11041																																						MH) JOL.	l	

#### Table 1-5 habilitation and Replacement Pr

					Con	eral Infor	mation												04-	uctural	Defect (	<sup>a</sup> ndlan	G	ravit	y Sew	rer Re	habil	litation	and R	epiac	emen			onal and	Maint							Con	struction	o Festur		<b></b>				8		T	<u> </u>
jact No.	D No.	pection No.	renset DVD No.	D Watched? (Y)	Location	CCTV		Ma	nhole	action of Camera	Cire	Pip	pe Atlas	CCTV Length	Crack	1 5		oken H B	tole	Joint		Collected Black	o Surface Damage			al Structural Defects	al Structural Defect Scor uctural Defect Index		osita D F Other			Ro Tap (T)	ots (R)	dlum	Ball (I	Infiit		Obstaci 65 OB Other	vermin C B	al O&M Defects al O&M Defect Score	18	Tap Lateral) T D[BI]BD		_ine L	Intru Ing Sea Mata Isi			orthy	ndition Ranking Deverse Setury Commister	Reverse Set-up Complete			ject Replacement Cost (\$)
a d	MQ	1881	Rev		Street Name	Date	Pipe ID	Start	End	Dia	Size (In)	Mat	(ft)	(ft) Grade						MLM	L		5 2				<u>s</u> 5		2 2		1 1 3	2 2	243	3 3	544	4 5 3		CZ %		10 10 10	8							P4	8 1	2	Commente	Minimum Recommendations	<u></u>
G-23	G312-055	6		Y	25TH ST	1/12/09	20053- 20054	20-053	20-054	4 U/S	6		95	109	1 1	1				1 2						6	19 3.2	2	1 1	2	13 1	1 1	1							21 27	7 1.3						5	Severe	23	(D/ siz	6.1' JOL (D/SH). 108' JOL (SL). Both JOL looks like e change. Not clear. DSZ DNF	Spot repair 106' to 108' to	39,876
G-24	G312-059	7		Y	MARINE AVE	1/16/09	21014- 21015	21-014	21-015	5 U/S	6	VCP	235	260		1	1			17 2						21	68 3.2	2			19	1								20 2	1 1.1						2 5	Severe	24	JO	2.7' JOL (D/SH). 205.5 IL (D/SL).	Spot repair 142.7' & 205.5'	94,806
G-25	G312-063	11		Y	18TH ST	1/27/09	22014- 22015	22-014	22-015	5 U/S	6	VCP	218	66				2		62						10	38 3.8	в		1	3		2							6 1 <sup>.</sup>	1 1.8		1			1		Severe	25	x (D/ 66.	JOL (D/SL). 66.2' JOL /SL). 7' & 9.1' BPVV. .2' MSA (JOL). U/S MH is 20.	7' 10 9.1' & 64.6' 10 66.2' Spo repair	t 79,282
G-26	G312-052	2		Y	17TH ST	1/7/09	15025- 15026	15-025	15-020	6 D/S	8	VCP	247	250					$\square$	52						7	25 3.6	6		1	23 1	1	1 3		1				Ш	31 42	2 1.4							Severe	26	jo	L (D/SL).	Spot repair 123.7' & 245.5' JOL	121,354
G-27	G312-071	з		Y	17TH ST	2/10/09	22008- 22009	22-008	22-009	9 U/S	6	VCP	226	194	1	4	1	1		23 1			6			37 1	09 2.9			3	6		1 1		3					14 26	6 1.9					1		Severe	27	2.4 x (Ca MH Da	2' BPVV, 8.5' BPSV, Also I' JOL (D/SH). 194.4' MSA amera can't climb). U/S I is a CO. Surface mage = SAVZ		82,413
G-28	A315-015	1		Y	19TH ST	5/29/09	22025- 22026	22-025	22-026	6 D/S	6	VCP	200	227	2 15	2		1		1						21	33 1.6	Б		4	31		5	2 1	1					44 6:	3 1.4					1	5	Severe	28	22 x Clo (JC	7 JOL (D/SH) & BPVV. ose to end MH. MSA 227 DL)	Spot repair	82,742
G-29	A315-019	8 A31	15-021 2	2	17TH ST	6/4/09	22011- 22012	22-011	22-012	2 D/S	6	VCP	112	77	1 2	1	1			2 1						8	23 2.9	•					1							1 :	3 3.0					2		Severe	29	BP	.3' MSA (JOL (D/SH) & 'SV)'. Close to end MH. SA 74.3' (3' from D/S MH) JL		40,824
G-30	G312-003 Ph 2	1			MANHATTAN AVE	7/9/09	14052- 14053	14-052	14-053	3 D/S	8	VCP	223	224						121						4	14 3.5	5 1	9										Ш	19 34	8 2.0					П		Severe	30	10/1	.6' (JOL D/SL). 38.4' (JOL SH).	Spot repair	108,621
G-31	G312-006 Ph 2	9		Y	4TH ST	7/14/09	10135- 10136	10-135	10-136	6 U/S	6	VCP	130	3			1			1						2	10 5.0	°		1	1									2 :	3 1.5					1		Severe	31	X MS C/0		Spot repair	47,385
G-32	G312-071	1		Y	39TH ST	2/10/09	23004- 23005	23-004	23-005	5 D/S	8	VCP	281	287	1	1		1		1 1						6	20 3.3	3		1	19 1	1	1 1		1					25 3	5 1.4							Severe	32	Sa	4.4' JOL (D/SH) & BPVV. me M313-002 #7 sleted).	Spot repair	139,482
G-33	G312-053	6 G31	12-053 1	1 Y I	32ND ST	1/8/09	19031- 19053	19-031	19-05	3 D/S	6	VCP	127	139	1	5		1		3 1						11	35 3.2	2	1		1									2 :	3 1.5			3		2		Severe	33	JO	.3' Small BPVV & 134.2' JL (D/SH). 134.8' MSA DL).	Spot repair 17.3' BPVV & 134.2' JOL	50,775
G-34	G312-011 Ph 2	7		Y	1ST ST	7/21/09	12025- 12026	12-025	12-020	6 D/S	8	VCP	100	107		5 1	4 1			1	1					13	45 3.5	5		2	10			1						13 1	5 1.2	1						Severe	34	98 JS	.7' JOL (D/SL). 102.1' L. 77.3' BPSV.	Spot repair	51,953
G-35	G312-054	3		+ +	ALMA AVE	1/9/09	20021	20-019			+ +		182	195	$\downarrow$	4	2		$\square$	9 1		$\square$		$\downarrow$			52 3.3		ļ	3	6 1								$\left  \right ^{1}$	20 30	0 1.5						╺┼╌╂	Severe	-+-		.7' JOL (D/SH).	Spot repair	70,932
G-36	G312-042	2		+ +	INGLESIDE DR	12/18/08	10132	10-129	-	2 U/S		-	255 337	257 336		5	1	$\left  \right $	++	10 1				_	9	L L	72 2.8 50 2.9	++-			29 1	9							╎╎╽	40 5	2 1.3		┝┼┼╴				_	Severe Severe		33	5.3' JOL (D/SH). 6.1' JOL (D/SH). Close to	Spot repair 125.3' JOL	93,786 122,508
G-37 G-38	G312-042 G312-028	4		-+	VISTA DR	11/26/08	10122 10143-	10-121	10-12	2 U/S 6 U/S	<b>∲</b> ₿		233	60	4					7 1				+	+		26 3.3		+		10	ġ.			╢			┼┼	┝┼╂	10 10	0 1.0	++					++	Severe	-	1011		57' Spot repair JOL (D/SH)	84,808
G-39	G312-011	11		Y. 1	8TH ST	11/3/08	10146 04054- 04056	04-054		6 D/S	}}		252	264		╁┼			+	2 1	$\left  \right $		1				13 3.3		+	1										1	1 1.0					╉╂	╶┼╌╂	Severe		- T	3.6' JOL( D/SH ). SAVZ	Spot repair 233.6 JOL (D/SH	) 128,498
G-40	G312-008 Ph 2	11 G31 P	12-012 Ph 2	4 Y	1ST ST	7/16/09	10056	10-056	14-100	0 D/S	6	VCP	20	8			1			1						2	9 4.5	5			1			1						2	4 2.0			1		2		Severe	40	& 1 (D/ 16	MSA (Camera doesn't fit) 16' (4' From D/S MH) JOL /SH). No inspection 4' to '. Pipe ID was 10056- 058A.	Spot repair	7,290
G-41	G312-027	11		Y	BOUNDARY PL				10-014	4 D/S	8	VCP	274	278					$\Box$	11						2	8 4.0	0 2			7		Ш						Ш	9 1	1 1.2							Severe	41	- 1	3.9' JOL (D/SH).	Spot repair 103.9' JOL (D/SF	() 134,962
G-42	A315-013	3		Y	MARINE AVE	5/27/09	21024- 21025	21-024	21-02	5 U/S	6	VCP	270	3						1	-					1	5 5.0	D												0	0.0					1		Severe	42	x (JC	5' JOL (D/SH). MSA DL). 336.1' MSA (JOL). S MH is a possible C/O.	Spot repair	98,415
G-43	G312-012 Ph 2	3		Y	1ST ST	7/22/09	14100- 10058	14-100	10-05	8 D/S	15	VCP	50	35						1						1	5 5.0	0 2												2	4 2.0		1					Severe	43	32 wa	.9' JOL (D/SH). Pipe ID as 10058A-10058.	Spot repair	31,985
G-44	G312-007 Ph 2		12-007 Ph 2	2 Y	VALLEY DR	7/15/09	10118- 10119	10-118	10-11	9 D/S	6	VCP	280	192		15	1	2		1						19	64 3.4	4		1	31 1									33 3	3 1.0					2		Severe	44	27 BP	7.7' MSA (JOL D/SL). 5.9' (4.1' from U/S MH) PSV. 168.6' BPVV. No spection 187.7' to 275.9'	Spot repair	102,060
G-45	G312-005	7		Y	FAYMONT AVE	10/24/08	02001- 02002	02-001	02-00	2 D/S	8	VCP	265	273		Π	1			1 1						3	13 4.:	3		3	44			1	$\prod$					48 5	0 1.0							Severe	45		4.4' JOL (D/SL). 270.3' VV (End of the sewer e).	Spot repair 244.4' JOL (D/SL	.) 132,484
G-46	G312-053	2 G31	12-053	3 Y	MANHATTAN AVE	1/8/09	18039- 18040	18-039	18-040	0 D/S	6	VCP	239	190	1	8 1				3 1						14	42 3.(	D 14		1 1	19 1		1	5	1					43 7	2 1.7					2		Severe	46	(Ci ) Ci (J) (J) (J) (J) (J) (J) (J) (J) (J) (J)	OL). No inspection 187.4' 236.04'. Same inspection 311-001 # 11 (deleted).	186.7' Spot repair to JOL	87,130
G-47	G312-019 -1	1 1		Y	FRANCISCO ST	8/5/09	10040		10-04			VCP	164	165	4 4	2	2		1	2 1				1	2	19	47 2.5	5			11 1	1	2		2					17 2	6 1.5		1					Severe	47	7 ( 15 RF fro	tme G312-008 Ph 2 # 6 & (deleted). 124.2' HSV & 4.5 JOL (D/SL). P=RPP. MSA 30.5' (134.1 m D/S MH) RBL.		80,141
G-48	G312-001	7		Y	HERRIN ST	10/20/08	01076- 01080	01-076	01-08	0 D/S	8	VCP	338	347	23				1	1 1						8	20 2.5	5 92		1		$\square$							Ш	93 18	5 2.0							Severe	48	24 HS	1.1' JOL (D/SL). 274.4' SV.	Spot repair 241.1' JOL (D/SL & 274.4' HSV	.) 168,399
G-49	G312-051	2 G31	12-053 1	1 Y	HIGHLAND AVE	1/6/09	19015- 19017	19-015	19-01	7 D/S	6	VCP	241	74		4		1		2 1						8	28 3.9	5		1	6				1					8 1	1 1.4					2		Severe	49	fit) fro fro BP 16	5' MSA (Camera does not & 169.48' MSA (71.3 ' om U/S MH) JOL. 70.8' m U/S MH JOL (D/SL) & VVV. No Inspection 2.5' to 9.48'	70.8' from U/S MH Spot repa to JOL & BPVV	<sup>lir</sup> 87,764
G-50	G312-039	5		Y	MARINE AVE	12/15/08	15054- 15055	15-054	15-05	5 D/S	8	VCP	302	307	1			1		1						3	12 4.0	0		1		$\prod$			$\prod$		$\square$		$\prod$	3	7 2.3	1	$\prod$					Severe	50		.3' JOL (D/SL) & 239.6' 2VV.	Spot repair 20.3' to JOL & 239.6' BPVV	149,105

#### Table 1-5 ewer Rehabilitation and Rep

										Table 1-5 Gravity Sewer Rehabilitation and		. <u></u>		0					<b>,</b>
Project No.	DVD No.	Inspection No.	Reversal DVD No. Reversal Inspec. No.	Contraction	CCTV Pipe ID Start End C	Pipe Atlas CCT Size Lengt Lengt (In) Mat (R) (R)		M SV VV SV	IJ VVOS MLML	attraction     attraction	Roots (R) Fine (F) Tap (T) Medium Bail ( her B L J C B L J C B L J C B L J	infiltration Obstaci		Construction Feature           ap           Line           T           L           BDD[L][U][R]RD[1]	Intrud 5 Ing 2 Seal 5 Mater 3 Mater 3 M D GT [Z] SA[CU] MC	Priority Condition Renking	2 Commenta	Minimum Recommendations	Project Replacement Cost (\$)
G-51	G312-036	2		Y NO STREET	12/10/08 15023- 15024 15-023 15-024 D		2 1 3 3 2 2 1 4 3	4 <u>5 5 5</u> 3 3	2	2         5         5         5         2         1         2         2         2         2         2         2         2           35         51         124         2.4         2 </td <td>2 2 1 1 1 1 3 2 2 2 4 3 3 3 5 4 4</td> <td>453424411</td> <td>2 2 1.0</td> <td></td> <td>22</td> <td>Severe 51</td> <td>71.5', 77' &amp; 106.7' BPVV). Sags. 139.1' to 199.7 MCU. Could not see any defect.</td> <td>Shouid Replace all line or Spot repair69' 1o 84' &amp; 104' 1o 107' for BPVV &amp; Fractures</td> <td>98,123</td>	2 2 1 1 1 1 3 2 2 2 4 3 3 3 5 4 4	453424411	2 2 1.0		22	Severe 51	71.5', 77' & 106.7' BPVV). Sags. 139.1' to 199.7 MCU. Could not see any defect.	Shouid Replace all line or Spot repair69' 1o 84' & 104' 1o 107' for BPVV & Fractures	98,123
G-52	G312-047	6		Y ROSECRANS AVE	12/29/08 18023- 18024 18-023 18-024 U	/S 6 VCP 163 18	1 1 1	1	1	4 14 3.5			1 1 1.0		1 5	Severe 52	180.7' JOL (D/SL) & BPVV. x MSA 180.7' (JOL). MH 18- 023 is a C/O	Spot repair 178' to 181' to BPVV & JOL	66,084
G-53	G312-056	5		Y 29TH ST	1/13/09 20005- 20006 20-005 20-006 D	/S 6 VCP 293 29	2 17 1	1	10 2	4 35 105 3.0	5 42 2 1		50 56 1.1			Severe 53	37.4'JOL (D/SL) & 100.4' JOL (D/SL). SAVZ	Spot repair 37.4' & 100.4' to JOL	106,361
G-54	G312-008 Ph 2	4		Y HOMER ST	7/16/09 10048- 10049 10-048 10-049 D	/S 8 VCP 250 20	6 1 1 1	1	1	5 16 3.2	10 1		11 12 1.1	1	1 5	Severe 54	151.7' BPVV. 205.8' JOL x (D/SL). 206.9' MSA (LD). D/S MH is a Burled MH	Spot repair	121,500
G-55	G312-003 Ph 2	6		Y THE STRAND	7/9/09 14068- 14069 14-068 14-069 D	/S 8 VCP 207 21	1 4 2 5	1	2 1	15 41 2.7	2 28 1 1		32 34 1.1 2	1 1		Severe 55	6.5' HSV. 209.2' JOL (D/SL).	Spot repair for HSV, Spot repair for JOL (D/SL)	102,643
G-56	A315-022	11 A3	315-022 1		6/29/09 11049- 11050 11-049 11-050 D	/S 6 VCP 250 10	5 3 1		2	6 15 2.5	7		8 11 1.4		2	Severe 56	56.3' MSA JOL D/SL & 200.9' (Reversal 46.5') JOL D/SL. No Reversal video between 56.3' to 200.9'.	Spot repair	91,125
G-57	G312-001- Ph 2	4		Y 14TH ST	6/10/09 11024- 11025 11-024 11-025 D	/S 8 VCP 305 30	7 2	1 2	4 1	10 37 3.7 6	11 1 1 1		20 31 1.6		5	Severe 57	(Channel of D/S MH).	Spot repair 4' Small BPVV, Spot repair Joi & BPVV 306.2'	149,056
G-58	A315-011	7 A3	315-011 9	Y 27TH ST	5/22/09 20048- 20049 20-048 20-049 D	/S 6 VCP 125 2	9	1	1	1 3 12 4.0			0 0 0.0	1	2	Severe 58	25.7' JOL ( DS/L) & BPVV. MSA 25.7' JOL & 121.5 (Due to angle of the pipe/incline). No inspection 25.7' to 121.5'	Spot repair	45,563
G-59	G312-003 Ph 3	1		Y 31ST PL	8/12/09 17071- 17072 17-071 17-072 D.	/S 8 VCP 350 36	4 1 1 1	1	1 1	6 21 3.5	6 58 2 4 2 3 6	1	82 121 1.5 1		1	Severe 59	144.3' JOL (D/SL) & 322.3'	Spol repair. Rool Treal & Cut	176,710
G-60	G312-002 Ph 3	7		Y 29TH ST	8/11/09 17012- 17013 17-012 17-013 D.	+ + + +	3	1	2 1	1 24 29 66 2.3 1	1 1 13		16 18 1.1	+ + + + + + + +	╶┼┽╉┼╷┥╉	Severe 60	5' BPVV. 83.5' JOL (D/SL). S= SAMZ 211 JOL (D/SL) & 189'		74,115
G-61	G312-017 -1	1		Y 3RD ST	8/3/09 10138- 10139 10-138 10-139 U	/S 6 VCP 400 38	2 1 1 13 1		33 1	17 68 187 2.8	3 53 2 1 2		61 73 1.2		15	Severe 61	Smail BPVV. S=SSSZ 301.8' JOL (D/SL). 356.7'	Spot repair	139,385
G-62	G312-002 Ph 3	6		Y 29TH ST	8/11/09 17011- 17012 17-011 17-012 D	/S 8 VCP 358 36	04		1 1 1	3 10 24 2.4	12942		18 33 1.8 1			Severe 62	HVV (End of the Pipe Line).	Should Replace pipe 163' to	174,863
G-63	G312-049	1		Y 33RD ST	12/31/08 18001- 18002 18-001 18-002 U	/S 6 VCP 408 35	7 8 2		29 2	27 68 179 2.6 3	6 18 2 2 3 2 2		38 61 1.6			Severe 63	78.3' & 79.9' JOL (D/SL). SAVZ	Spot repair 78'10 80'10 JOL.	148,785
G-64	A315-001	13 A3	315-004 9	Y PARKVIEW AVE	5/6/09 25027- 25028 25-027 25-028 D	/S 8 VCP 237 24	1 2 4 2 1 1	1 2	1	5 19 47 2.5 17			17 34 2.0	1	2 5	Severe 64	143.4' to 153.1' BPVV. 201.3' BPSV. 203' MSA (BPVV). 133' to 163.8 continuous cracks, fractures & BPVV.	133' 1o 164' Reline Pipe & 201.3' Spot repair	117,029
G-65	A315-012	13 A3	315-013 2	28TH ST	5/26/09 19039- 19045 19-039 19-045 D	/S 6 VCP 130 10	9 2	1	1	4 12 3.0	1 1 10		12 13 1.1		2 5	Severe 65	105.9 BPSV (Missing Pipe). 127' JOL (DS/L). MSA 105.9' (BPSV). There is no inspection 105.9' to 127'	Spot repair	47,385
G-66	G312-061	4		Y MARINE AVE	1/23/09 21016- 21017 21-016 21-017 D	/S 6 VCP 250 25	6 6 6 2	4 1	911	30 83 2.8	14 4 1		19 25 1.3	1		Severe 66	JSL. 235.4' BPVV.	Spot repair for JOL, JSL & BPVV	93,348
G-67	G312-019 -1	3		Y 5TH ST	8/5/09 10131- 10132 10-131 10-132 U	/S 6 VCP 258 25	5 2 3 2	1	9 1	2 2 2 22 57 2.6	1 15 2 1 3		22 33 1.5			Severe 67	S=SSSZ. RP=RPR	Spot repair	93,093
G-68	G312-031	6 G3	312-031 7	Y MANHATTAN BEACH BLVD	12/3/08 11046- 11047 11-046 11-047 D	/S 6 VCP 278 30	5 1 11 2		7 1	13 35 91 2.6	20 1 1 1		23 28 1.2		2	Severe 68	272' JOL (D/SL). SAVZ. MSA 272' (33' from D/S MH) JOL	Spot repair 272 to 274' to JOL (D/SL)	111,173
G-69	R311-002	10 R3	311-002 11	Y 31ST ST	12/30/08 19001- 19002 19-001 19-002 D	/S 6 VCP 284 31	9119	1	6 1	19 57 3.0	3 42 3 1 6		55 75 1.4	1	2	Severe 69	8.1' JOL (D/SL). MSA 262.3' (56.8' from D/S MH) TBi	Spot repair 8.1' to JOL	116,312
G-70	G312-006 Ph 2	8 G3	312-006 4 Ph 2 4	Y 4TH ST	7/14/09 10136- 10137 10-136 10-137 D	/S 8 VCP 250 25	4 1 11	1	5 1	19 58 3.1 8	2 26 4 2		42 56 1.3	2	2	Severe 70	226.3' JOL (D/SL). MSA 237' (17.1' from D/S MH) LR. inspection complete	Spot repair	123,493
G-71	G312-001- Ph 2	7		Y ARDMORE AVE	6/10/09 11018- 11021 11-018 11-021 D.	/S 8 VCP 310 30	6 1 15 1	2	2 1	22 69 3.1 5	3 20 1 2		31 44 1.4			Severe 71	5' JOL (D/SL).	Spot repair	148,910
G-72	G312-003 Ph 2	10		11TH ST	7/9/09 14055- 14056 14-055 14-056 U	/S 6 VCP 131 S	742	1	8 1	16 47 2.9			0 0 0.0			Severe 72	35.7' JOM (D/SL). U/S MH is a C/O. 96.6' MSA (Camera flips over at JOM)	Spot repair	47,750
G-73	G312-035	13		Y 10TH ST	12/9/08 11130- 11131 11-130 11-131 U	/S 6 VCP 139 14	3 1 3		5 1	8 18 47 2.6	1 3		4 4 1.0			Severe 73	112.2' JOL (D/SL). SAVZ	Spot repair 112.2' to JOL (D/SL)	52,051
G-74	G312-004	10		Y 18TH ST	10/23/08 01221- 01222 01-221 01-222 U	/S 8 VCP 278 28	2 3 1 7 1		21	15 41 2.7 4			14 22 1.6			Severe 74	194.7' JOL (D/SH).	Spot repair 194.7' 1o JOL (D/SL)	136,955
G-75	G312-006 Ph 2	10		Y 3RD ST	7/14/09 10140- 10141 10-140 10-141 U	/S 6 VCP 250 11	1 2 1		4 1	8 25 3.1	2 11 1 2		16 20 1.3		1	Severe 75	x 111.1' MSA (JOM).29.2' JOL (D/SL). U/S MH is a C/O		91,125
	G312-004 G312-002	1		Y 9TH ST	10/23/08 01161- 01162 01-161 01-162 D		╏╏┼┼┼┼┤		31	1 8 20 2.5	5 6 2 1		14 19 1.4		<del>╶┟╶┟┠┊┊┊┨</del>	Severe 76	95.3' JOL (D/SL). SAVZ	Spot repair 95.3' JOL (D/SL) & Line Down	168,253
G-77 G-78	Ph 2 G312-001	5		AVE	7/8/09 14048 14-048 14-050 D 8/10/09 08069 08-069 08-070 D		╂┈╂╌╁╶┽┼┤		3 1 1	5 15 3.0 2			2 4 2.0		┽┼╂┊┼┨	Severe 77	207.8' JOL (D/SL).	Spot repair	176,637
G-78 G-79	Ph 3 G312-003	7		Y LAUREL AVE	7/0/02 14069- 14 050 14 070 0	╺┼╍╍┼╺╌┼──╴┼──╸	0 2 1		4 1	5         13         2.6           4         14         39         2.8	2	┼┼┼┼┝┝┝┝	32 38 1.2 2 2 1.0		┽┼╂┼┼╂	Severe 78 Severe 79	83.1' JOL (D/SL). 16.5' JOL ( D/SL).	Spot repair Spot repair	111,537 15,017
	Ph 2 G312-045	3		Y 30TH ST	14072	/S 8 VCP 347 35	+ $+$ $+$ $+$ $+$ $+$ $+$ $+$ $+$ $+$	┝┼┾┼┥	2 1	5 15 3.0			7 15 2.1	┼┼┼┼┼	<del>╶╎╺┼┨╶┼╶┦╸┨</del>	Severe 80	320.3' JOL (D/SL) .	Spot repair 318' to 320.3' to	174,523
G-81	G312-066	3		Y SAUSALITO CIR	2/3/09 24093- 24094 24-093 24-094 D	/S 8 VCP 135 14	4		2 1	3 11 3.7 8			8 16 2.0			Severe 81	51.2' JOL (D/SL).	Spot repair	70,178

																					Gravity	/ Sewe	er Reha	Tabi bilitatio		epiacer	nent P																		
Project No.	DVD No.	Inspection No. Reversal DVD No.	Reversal Inspec. No. DVD watched? (Y)	Gen	CCTV Date		Man	hole End	Direction of Camera	ze n) Mat	Length (ft)	(11)		F	N SV V	H V SV V	/V 0 M L	nt S V M L	T Deformed	v X Collapsed Pipe w Surface Damage	- I	Segs	Total Structural Defects Total Structural Defect Scot	AGS		BLJ	CBL	Roots (F 9 (T) I J C B	i) fedium	Ball (B) B L J C	Infiltrati	0			olad Ma	Tap teral) T BI BD D		ini ii S Ma I		Miscellaneous	Priority	Condition Ranking No Raverae Set-up Completed	Comments	Minimum Recommendations	Project Replacement Cost (\$)
G-82	G312-070	3	-   <sub>1</sub>	33RD PL	2/9/09	17065- 17066	17-065	17-066	D/S	B VCP		Grade	2 1 3				2 1	1 2 1	5 5			2	3 11		2 2	5	132	1	1	1				8 14	1.8			++			Severe	82	164.9' JOL (D/SL).	Spot repair	81,113
G-83	A315-012	11	T Y	27TH ST	6/26/00	19037- 19037A	19-037	19-037A	D/S	6 VCP	120	125	1 1				11		+-+				4 12	3.0	1	9								99	1.0	1			1		Severe	83 x	124.7' JOL(D/SL). MSA 124.' (JOL).	Spot repair	45,453
G-84	G312-022	4	Y	SEPULVEDA BLVD	11/18/08	08001- 08002	08-001	08-002	D/S	B VCP	142	112		1			11						3 10	3.3 8		1 1								10 18	1.8						Severe	1	7' JOL (D/SL). Pipe ID was 08001-08001A.	Sportepair / JOC (U/SC)	54,626
G-85	G312-010	2	Y	LONGFELLOW DR	10/31/08	04025- 04026	04-025	04-026	D/S	B VCP	210	212					11			1			3 10	3.3		2 23								25 25	1.0								121.5' JOL (D/SL). SAVZ	Spot repair 121.5' to JOL (D/SL)	103,032
G-86	G312-028	2	Y	6TH ST	11/26/08	10073- 10075	10-073	10-075	D/S	B VCP	225	230	1				1 1						3 10	3.3		1 1							4	6 6	1.0						Severe	86	34.1' JOL (D/SL).	Spot repair34' 1o 36' 1o JOL	111,537
G-87	G312-055	7 G312-0	055 8 Y	25TH ST	1/12/09	20054- 20063	20-054	20-063	D/S	5 VCP	133	67	1				1						28	4.0		16								16 16	1.0				2		Severe	87	64.1' JOL (D/SL). 64.5' MSA (JOL). 130.85' (2' from U/S MH) MSA (Camera does not fit). No inspection 64.5' 10 130.85'	Spot repair	48,424
G-88	G312-024	12	Y	29TH ST	11/20/08	08002 ]	08-061	08-062	D/S	B VCP	235	241	1				1						2 8	4.0		4				1				58	1.6	1	1	1			Severe	88	238.2' JOL (D/SL). Close to End MH.	Spot repair 238.2' JOL (D/SL)	117,223
G-89	G312-013	10	Y	SHORES PARKING LOT	11/5/08	05009- 05010	05-009	05-010	D/S	B VCP	300	316		1	1		1						3 11	3.7	_	56	1							57 57	1.0						Severe		5' JOL (D/SL).	Spot repair 5' JOL (D/SL)	153,673
G-90	A315-021	3	Y	13TH PL		14025- 14026	14-025	14-026	u/s	6 VCP	168	79	2		$\square$	11	1		$\downarrow$			$\downarrow$	_	2.3	_	2				1				36	2.0		+++						78.8' MSA (JOL D/SL). U/S MH is a possible C/O 167.5' (JOL D/SL). 167.5'		61,236
G-91	A315-013	9 A315-0	013 10 Y	23RD ST	5/2//09	15071- 15072 15053-		15-072	<u> </u>		┝────┾	253		$\left  \right $	$\square$	$\downarrow$	1		$\downarrow$		_⊥	+		5.0		3 46			2	5	$\left  \right $			56 75	1.3		╶┼┼┾		2				(85.8' 1rom D/S MH) JOL.	Spot repair	131,220
G-92	G312-039	4		MARINE AVE	12/15/08	15053- 15054 12015-			+ +-	B VCP	$\vdash$	316	++	$\square$		+	1			_		┾╌╄		5.0	_		2				$\left  \right $		╉╌╫╂╶	4 4	1.0 1								60.7' JOL (D/SL).	Spot repair 60.7' to JOL	153,333
G-93	G312-033	7		2ND ST	12/5/06	12016				B VCP	<b>└──</b> ↓	94	++	$\left  \right $		+	1		+		$\square$	++		5.0				┝╌╄╌┠╌		┝╋╋╋	┼┼┼┤		┼┼┼		1.0	++		1				93	17' JOL (D/SL).	Spot repair 17' JOL (D/SL)	45,490
G-94	A315-016	12	+	19TH ST	0/1/09	22055	22-028	22-055	D/S	5 VCP	143			┼┼┼	++	++	-	++	++	+	++	++	1 5	5.0		╋╋	$\left  \right $				$\left  \right $	╞┼┼	┼┼┼		0.0		┽┼┼		┼╂╌╵			-	3.5' MSA (JOL). 2' JOL (D/SL). 7.5' MSA	Spot repair	52,124
G-95	G312-009 Ph 2	4	۱ I	1ST ST	7/17/09	10148- 14100	10-148	14-100	D/S	6 VCP	20	8					1						1 5	5.0										0 0	0.0	1					Severe	95 x	10058B.	Spot repair	7,290
G-96	A315-024	8	,	36TH ST	7/15/09	19046- 19048	19-046	19-048	u/s	6 VCP	285	140	2 6 1	1		1	1						12 23	1.9	7									7 14	2.0				1		Severe	96 x	140.1' MSA (BPSV). Following two pipes segments seem to have a JOL. MH 19-046 is possible CO	After 140.1' , Replace or Spot repair 2 or 3 pipes	103,883
G-97	A315-012	12	v	28TH ST		19038- 19039	19-038	19-039	u/s	6 VCP	175	3			1								1 5	5.0										0 0	0.0				1		Severe	97 x	2.5' BPSV (Missing Pipe). 2.5' MSA (BP SV). U/S MH is a CO	Spot repair	63,788
G-98	A315-018	3		13TH ST	6/3/09	14013- 14016	14-013	14-016	D/S	8 VCP	296	300	1 62	2 1			2						66 195	3.0		2 43			2	1 1				49 61	1.2						Severe	98	Continuous Cracks (Multiple). 278' to 284' possible abandoned MH (MH14-035). Bottom part is in poor condition.	Reline or Replace pipe 278'10 284'	145,606
G-99	G312-056	9		VISTA DR	1/13/09	20009-	20-009	20-012	D/S	8 VCP	130	129	1 2	2 2	1 2	1							9 30	3.3 3		6		1						10 14	1.4						Severe	99	2' BPVV. 121.5' & 122.5' BPSV.	Spot repair 2' to 4' BPVV & 119' to 128' BPSV and	62,840
G-10	G312-004	7	++,	AVIATION	10/22/08	01187-	01-187	01-189	D/S	8 VCP	340	343		+++	2 2	+					1	┼╂	7 23	3.3			╏╏╏╏							0 0	0.0	1				2	Severe	100	223.8' & 226.8 ' (two sides) BPSV. Pipe iD was 01187-	Fractures 220' 1o 229' Fractures Multiple & Broken Pipes. Spot repair	166,601
G-10 <sup>-</sup>	G312-072	7		20TH ST	++-		22-029	22-030	U/S	6 VCP	161	3				1				1	1		2 7	3.5										0 0	0.0					1	Severe	101 x	01089. 2.5' BPSV (Missing Pipe). 2.5' MSA (BPSV). U/S MH is a CO. Surface Damage =	220' 10 229' Replace pipe or Spot repair	58,612
G-10	G312-067	5	,	19TH ST	2/4/09	22023- 22024	22-023	22-024	U/S	6 VCP	330	4			1								1 5	5.0				1					2 15	3 12	4.0						Severe	102 ×	SAMZ 4.2' BPSV. 4.2' MSA (OBM). OBM & OBN. U/S MH is a CO.		120,278
G-10	G312-037	7	`	16TH PL	12/11/08		14-003	14-004	u/s	6 VCP	105	68		2 2	1	1	4						16 49	3.1		4			4					8 16	2.0	1			1		Severe			Spot repair 21' to 27' tor HSV & fractures	38,269
G-10	G312-005	4		' 19TH ST	10/24/08		01-224	01-225	u/s	8 VCP	278	285	1 2 3	3 2	4	2			$\downarrow$			$\downarrow$	14 43	3.1		122		$\square$					$\downarrow\downarrow\downarrow\downarrow$	6 10	1.7 3				11-		Severe	104	17.4' & 144.1' BPVV with continuous fractures. 6' MSA (BPVV). 4' to 6'	Spot repair or Replace pipe 13' 10 19' & 140' 10 146'	138,413
G-10	G312-052	5		38TH PL	1/7/09	18050- 18051	18-050	18-051	u/s	6 VCP	140	6				2							2 10	5.0										0 0	0.0				1		Severe	105 x	6' MSA (BPVV). 4' to 6' BPVV (Missing Pipe). U/S MH is a possible C/O	Spot repair 2' 1o 6' to BPVV	51,132
G-10	G312-057	7	$+\uparrow$	MANHATTAN	1/14/09	I	20-044	20-046	D/S	6 VCP	245	256	2 10	;;;;	4	++	4		1	+	++	┼╂	21 65	3.1		1 3								4 4	1.0			$\dagger \dagger$			Severe	106	157.9' Vertical Deformed Pipe.	Spot repair 157.9' to vertical deformed pipe	93,421
G-10	G312-051	3		33RD ST	1/0/00	19016- 19017	19-016	19-017	u/s	6 VCP	219	4			2	2							4 18	4.5										0 0	0.0				1		Severe	107 x	4' MSA ( BPVV). U/S MH is Poinsettia Pump Station.	Spot repair 0' to 6' BPVV	79,785
G-10	G312-054	1	Ţ,	ALMA AVE	1/0/00	20010	20-018	20-021	u/s	6 VCP	408	407	1 6		4	2 1	13						27 90	3.3	1	4 62		2	3	2				74 89	1.2				1		Severe	108	97' & 110.1' BPVV. 186.1' HSV. U/S MH is a possible C/O.	+	148,169
G-10	A315-001	4		, MAGNOLIA WAY	5/6/09	25015- 25016	25-015	25-016	u/s	8 VCP	303	303				1							1 5	5.0	26									27 56	2.1						Severe	109	103.1' HSV & infiltration Runner at the same point ISZ=ISSRH	Spot repair	147,258
G-11	A315-005	8 A315-0	005 9	, ROSECRANS PL			18-048					16	,		1									4.0		1								1 1	1.0				2		Severe	110 ×	15.8' BPSV (Missing Pipe). 15.8' MSA (BPSV). There is no inspection 15.8' to 200' (Due to angle of mouth of the pipe).		72,900
G-11	G312-005 Ph 2	1		VALLEY DR	7/13/09	10115- 10116						347	1 1 8		$\square$	1	6	ЦĪ				μĪ	17 50		8	2 28	2	Ш	ЦТ			Ш		40 48	1.2					1	Severe	_	103.7' HSV (Missing Pipe).	Spot repair	126,627
G-11	G312-054	4		25TH ST	1/9/09	20021- 20022	20-021	20-022	D/S	6 VCP	109	51		•	1	1							6 21	3.5		7	ΊĪ		2					9 13	1.4						Severe	112	34.7 BPVV (Missing Pipe).	Spot repair 34' to 38' for BPVV & Fracture	18,480

1         1        1         1        1        1         1        1        1        1        1 <t< th=""><th></th></t<>	
	Itum Recommendations
No.       N	epair 70' to 78' for 109,642 & Fractures
b         b	epair 74.8' BPVV & 124 310
n         n        n        n        n        n      <	epair 46,170
	epair 127,575
N 1000         N 1000        N 1000        N 1000        N 1000        N 1000        N 1000         N 1000         N 1000         N 1000         N 1000         N 1000         N 1000        N 1000          N 1000        N	Repair 5,285
Norme	epair 41,553
b         v        v        v         v        v        v        v        v        v	epair 51,856
1 1         1 1 <td></td>	
Line       V	ce pipe 347' to 350' 171,461
11000         1 <th1< th="">        1         <th1< th=""> <th1< th=""></th1<></th1<></th1<>	epair 63,788
011/200         0         1         011/200         0100         1000        1000        1000        1000        1000     <	epair 42,391
a bit sin	epair 169.6' to 173.7' 115,911
c - c - c - c - c - c - c - c - c - c -	epair 70,020
Gir 2         Art bools         Fe         Art bools         Fe         Art bools         Fe         Fe       Fe        Fe	epair 81,065
A 15 000         S         V         S 1000        1000        1000        1000        1000        1000        1000        1000        1000        1000       1000       1000       1000	epair 54,675
A 3 1 0 0 3 A 3 1 0 0 3 A 9 Waxxx B 4 0 0 200 0 200 0 200 0 200 0 200 0 20 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	epair 40,095
Gala       All-Gala       All	epair 133,954
Bry       2       1       Bru       1       1       1       2       1       1       2       1       2 <td>epair 76,108</td>	epair 76,108
C 132         C 312-008         12         V 2 155 T         102008         03006         05.08         0.5.8         V P         188         1         1         1         2         1         1         2         Main         12         16         13         1         1         2         Main         12         16         13         1         1         2         Main         13         1 <th< td=""><td>epair 264' to 267' 167,792</td></th<>	epair 264' to 267' 167,792
6 134         0 312-05         5         1         9 315 TS T         1/5/0 19005         19-05         19-05        19-05        19-05        19-05	epair 110' to 113' 10 96,374
G-138       G312-007       4       Y       3151       11/10/6       2007       12/2       1       5       11/1 <th< td=""><td>epair 100' 1o 103.2' 49,135</td></th<>	epair 100' 1o 103.2' 49,135
G-136         G312-002         8         v         2 ND ST         1021/06         01083-01085         01-083-01085	Spot repair to BP VV 106,179
G-13         G312-002         8         Y         2ND ST         10/2108         01085         01-085         01         8         VCP         356         2         9         13         4         3         4         1         7         47         102         6         15         30         1         5         10         1         7         47         102         6         15         30         1         1         7         47         102         6         15         30         1         1         7         47         102         6         15         10         1         1         7         47         102         6         15         30         1         1         7         47         102         6         15         30         1         1         1         7         47         102         6         15         30         1         1         1         2         1         1         1         2         1         1         2         1         1         2         1         1         1         1         1         1         1         1         1         1       1       1         1        <	epair 3.5' to BP VV 91,807
Gale         Gale         I         Gale         Gale<	93' Replace pipe for .199.9' Spot repair for 172,287 BPVV
Gale         Gale         I         Gale         Gale<	epair 23.3' 1o BPSV 174,474
G-140     A315-007     8     Y     THE STRAND     5/18/09     19047     19-047     19-048     U/S     8     VC     200<	epair 2' to 3' for BPVV 44,287
G-140 A315-007 B V THE STRAND 5/18/09 19047- 19048 19047 19-048 U/S 8 VCP 200 206 3 1 1 1 1 0 1 1 1 0 1 1 1 0 1 1 1 0 1 1 1 0 1 1 1 0 1 1 1 0 1 1 1 0 0 1 1 1 0	epair 8.9' & 90' to BP 66,339
	epair 100,067
G-14 A315-002 2 Y HOMER ST 5/7/09 23027- 23028 23-027 23-028 D/S 8 VCP 350 357 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
G-142 G312-037 10 Y 14TH S1 12/11/08 14064 D/S 6 VCP 116 116 4 1 1 1 1 1 1 3 1 1 1 1 3 3 3 3 3 1 1 9 9 1 1 1 1	epair 103' to 106' for 42,100
G-143 G312-030 2   Y AVE 12/2/08 12/00 12-008 U/S 8 VCP 236 268 15 2 1 1 9	epair 261' 10 264' 130,348 Root Treat & Cut 130,348
G-144 G312-00 13 Y 3RD ST 10/31/08 04011 04-008 04-011 0/S 8 VCP 332 350 2 2 1 1 1 2 3 12.6 2 40 1 1 1 4 4 6 6 6 1.3 6 1.3 6	epair 89.2' BPVV & 169,906 epair 348.1' HVV
	epair 196.8' BPVV, epair 1o Surface 148,619 ge
	epair 160' to 162' 83,944
G-147 G312-059 3 G312-059 4 Y 21ST PL 1/16/09 22042 22-043 D/S 6 VCP 253 261 2 6 1 1 5 44 2.9 2 7 1 1 1 5 Spot re	epair 95,244
G-148 G312-059 1 Y 21ST PL 1/16/09 22040- 22041 22-040 22-041 U/S 6 VCP 253 257 1 2 8 1 8 1 8 20 57 2.9 1 1 5 1.7 1 1 9 15 1.7 1 1 1 9 15 1.7 1 1 1 9 15 1.7 1 1 1 9 15 1.7 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	epair 93,713
G-149 G312-004 Ph 2 5 V 19TH ST 7/10/09 14001- 14002 14-001 14-002 D/S 8 VCP 250 237 2 6 1 1 1 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0	epair 115,085
G-150 G312-010 Ph 2 1 V 10TH ST 7/20/09 11083- 11-083 11-084 D/S 6 VCP 100 128 2 5 3 1 1 1 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	epair 46,729

																						Gr	avity S	ewer	Rehabi	itation	and Re	placer	nent Pi	roject	ts																		
					Ge	neral info	rmation												Structu	ral Defec	t Coding									Оре	erational	and Mai	ntenan	ce				- 1	Cor	struction	Feature	18		Т		3			<u>s</u>
No.	,	ion No.	i DVD No.	Il Inspec. No. Itched? (Y)	Location			Ma	inhole	n of Camera		Pip	28		Crack	Fracture			ol, e	pint	Deformed	Surface Damage	roum Hepaur allure	ructural Defects	ructural Defect Scol rai Defect Index	Depo	osite			Roota				nfiltratio		ş	AM Defects AM Defect Score	efect Index	Tap (Lateral)	L	Ine	Intrud Ing Seal Mater Ial	Miscellaneous		on Ranking	erse Set-up Complet			Replacement Cost (
털	2 ×	1 1 1 1	nsua,	i i i i i i i i i i i i i i i i i i i		0071				ctiq	0		tias C		C C	F						(SF	비밀		동물		D E Other				Medius				OB V Othe		8 8	l ŝ la				IS DIGTIZ	SA CU	MC	Ê	i A			<u> </u>
P 2	N N	불	Rev	P A	Street Name	Data	Pipe ID	Start	End	Dia	Size (in)	Mat	ingth Le	Ingth L (ft)				1 34	ML	ML	<u>+ - + -</u>		ㅋ희		불봉	AGSIA									TCIZI	× –	월 월	₹ľ							Ĕ	2	Comments	Minimum Recommendations	2
									1				PACP G	irade 2	1 3	324	5 0	5 5	5 3 5	12	5 5 1	5 2	1	2		2 2	2	211	132	2 2	4 3 3	3 5 4	445	342	2 4 4	11													
G-151	G312-001- Ph 2	3		۲	14TH ST	6/10/09		11-023	3 11-024	D/S	8	VCP	330	336	1 2			1	3					7	21 3.0			1 16	3			1					21 24	4 1.1						Ма	ajor 15	51	335.6' BPVV (Channel of D/S MH ).	Spot repair 335.6'	163,247
G-152	G312-001- Ph 2	1		Y	15TH ST	6/10/09	11019-	11-019	11-020	D/S	8	VCP	255	260 1			1	Π	3					5	16 3.2	2		2 3	2		1	1 2					11 2	1 1.9						Ма	ajor 15	2	257.7' BPSV.	Spot repair 257.7	126,263
G-153	Various 1 (G312-015)		Various 1 (G312- 015)	6 Y	THE STRAND	7/30/09	12037- 12038	12-037	12-038	B D/S	8	VCP	235	259 2	3 11	1 :	1							2 21	55 2.0	3	2										2	4 2.0	1 1				2	М	ajor 15	i3	63.1' BPSV. Major Defect. D	Spot Repair	125,631
G-154	G312-007 Ph 3	4		۲	BELL AVE	8/18/09	17032- 80133	17-032	17-03	3 D/S	8	VCP	150	95				1	1					2	8 4.0			3	ПП		Ш	Ш	Ш				3	3 1.0						Ma	ajor 15	24	sewer line).	Spot repair	46,170
G-155	G312-005 Ph 3	2		Υ	36TH PL	8/14/09	17028-	17-028	3 17-029	D/S	8	VCP	235	243	1		П	1	$\top$	Ш				2	8 4.0		$\square$	3 4	ПП		1	2	Ш				10 1	8 1.8						Ma	ajor 15		sewer line)	Spot repair	118,292
G-156	G312-003	1		Y	NELSON AVE	10/22/08	01025-01026	01-025	5 01-026	B D/S	8	VCP	350	357			Π	1	1 3	Ш				5	i 19 3.I	3		5 4			5				ТП		14 24	4 1.7	1					Ма	ajor 15	<b>50</b>	(End of the sewer line)	Spot repair 248.6' HSV & 354.1' HVV	173,308
G-157	G312-005	3		Y	18TH ST	10/24/08	01222-01223	01-222	2 01-223	B D/S	8	VCP	278	284	48	2	$\square$	$\square$	3	Ш		1		54	159 2.	25 5	2		$\prod$		1			5	1		129 25	9 2.0						м	ajor 15	- 1 10	Most joints have DAE, IW & CM. SAVZ.	Reline	137,975
G-158	G312-030	5		Y	1ST ST	12/2/08	10010		12-02	3 D/S	6	VCP	259	263 2	57				2			33		95	251 2.0	5 2	$\square$	1 15	1		1	1					21 2	9 1.4				1		м	ajor 15		Continuous Crack Multiple & Surface Damage. SAVZ. ISSRH	Reline	95,718
	-				-		•			-		To	stal ft 3	1,032																																		Total	15,329,403

Table 1-5

Total ft 31,032 Total miles 5.9

						Ma	nhole Re	ehabili	tation	and R	eplacer	nent P	rojects	5								
																			Ranking			
Project No.	Inspection Phase	Session ID	Street	Manhole	Inspection Date	Surface Condition	Manhole Cover	Frame	Cone	Barrel / Wall	Rungs	Bench	Channel	Debris	Grease	Vermin	Odor	<b>Priority Score</b>	<b>Condition Ran</b>	Comment	Recommendation	Project Cost (\$)
																				Fractures in cone and barrel. Fracture seen in street pavement		
M-1	1	25	Highland Ave	19-015	1/12/09	Pavement - Concrete Collar	Good	Good	Failing	Failing	Good	Good	Good	No	No	No	No	24	1	surrounding manhole cover.	Replace	30,000
		00	1 - 1 - 01	04.000	44/04/00		0		Dese	Deen	E a Ular a	Deser	Desa	N.	NIE	0	N.,	00	0	Missing mortar. Corrosion at	line menhala	10,000
M-2	1	28	1st St	01-080		Pavement - Concrete Collar	Good		Poor	Poor	Failing	Poor	Poor	No	No	0	No	22			Line manhole	10,000
M-3	1	74	Duncan Ave	10-039		Pavement - Concrete Collar	Good		Good		Failing	Poor	Poor	No		No	No	18	3		Line manhole	10,000
M-4	1	90	Gates Ave	01-072	11/20/08	Pavement - Concrete Collar	Good	Good	Poor	Fair	Poor	Poor	Poor	No	Yes	0	Yes	15	4		Line manhole	10,000
							<b>.</b> .						_						_		Replace manhole	40 500
M-5	1	51	Curtis Ave	05-052		Pavement - Concrete Collar	Broken	Good		Good	Poor	Poor	Poor	No	No	No	No	14	5		cover and line	13,500
M-6	1	36	2nd St	01-086		Pavement - Concrete Collar	Good	Good		Good	Poor	Poor	Poor	Yes	No	0	No	11	6		Line manhole	10,000
M-7	1	109	18th St	06-244		Pavement - Concrete Collar	Good	Good		Good	Poor	Poor	Poor	No		No	No	10	7		Line manhole	10,000
M-8	1	23	8th St	04-054	12/1/08	Pavement - Concrete Collar	Good	Good	Good	Good	Poor	Poor	Poor	No	No	No	No	10	8		Line manhole	10,000
MO	4	50	Magdawa Ava	05 000	4.2/2/00	Devery anti-Concepto Coller	Cood	Coord	Caad	Caad	Deer	Deer	Deer	Nia	Na	Nia	Nia	10	0	Corrosion at bench and channel.	Line manhole	10,000
M-9	1	50	Meadows Ave	05-023		Pavement - Concrete Collar	Good	Good		Good	Poor	Poor	Poor	No	No	No	No	10	9			10,000
M-10	1	130	14th St	06-191	12/8/08	Pavement - Concrete Collar	Good	Good	Good	Good	Poor	Poor	Poor	No	No	No	No	10	10		Line manhole	10,000
M 4 4	4	201		05.040	4/22/00	Deveryont, Concrete Caller	Cood	Coord	Deer	Deer	Deer	Cood	Cood	Nia	Na	Nia	Nia	10		Corrosion and cracking in cone and	Lina manhala	10,000
M-11	1	291	Cedar Way	25-012		Pavement - Concrete Collar	Good			Poor	Poor	Good	Good	No	No	No	No	10			Line manhole	10,000
M-12	1	293	Village Dr	25-025	1/22/09	Pavement - Concrete Collar	Good	Good	Poor	Poor	Poor	Good	Good	No	No	No	No	10	12		Line manhole	10,000
M 40	4	50	Devial Ave	05.055	40/0/00	Deveryont, Concrete Caller	Draken	Coord	Cand	Caad	La:	Cood	Cood	Nia	Na	Nia	Nia		10		Replace manhole	2 500
M-13 M-14	1	53 47	Rowell Ave	05-055 05-020		Pavement - Concrete Collar	Broken		Good		Fair	Good Fair	Good Poor	No No		No No	No No	9			cover Line manhole	3,500 10,000
	1		Tennyson St			Pavement - Concrete Collar	Good		Good	-	Poor	Fair	Poor Poor	No		No		Ť			Line manhole	10,000
M-15	1	60	Voorhees Ave	05-005		Pavement - Concrete Collar	Good		Good		Poor		Poor	No		No	No No	9 9	-			10,000
M-16 M-17	1	181 46	Flournoy Rd 11th St	17-057 09-028		Pavement - Concrete Collar Pavement - Concrete Collar	Good Good	Good Good	Good Good		Fair Good	Poor Poor	Poor	No		Yes	No	9	16 17		Line manhole Line manhole	10,000
M-18	2	235	The Strand	12-030A		Pavement - Concrete Collar	Good	Good	Good		Poor	Good	Fair	No		No	No	9			Repair mortar	6,000
M-19	2	235 80							Good					No		No		9			Line manhole	10,000
	1	131	14th St	06-050		Pavement - Concrete Collar Pavement - Concrete Collar	Good	Good			Good	Poor	Poor			No	No	0	20			10,000
M-20 M-21	1	178	14th St	06-077			Good	Good	-	Good	Good	Poor	Poor	No No			No No	0	20		Line manhole	10,000
M-22	1	99	31st St Manhattan Ave	17-053 11-116A	-	Pavement - Concrete Collar Pavement - Concrete Collar	Good Good	Good Good	Good	Good Good	Good	Poor Good	Poor Good	No		No No	No	0			Line manhole Line manhole	10,000
M-23	1		35th Pl	17-049	0/4.4/0.0				Poor		Poor		-			~		0				10,000
10-23	3	25	5501 FI	17-049	8/11/09	Pavement - Concrete Collar	Good	Guu	Good	Guuu	raii	⊦aır	Poor	NU	No	0	No	0	23		Line manhole Replace manhole	10,000
M-24	1	284	The Strand	14-064	1/21/09	Pavement - Concrete Collar	Cracked	Good	Good	Good	NI/A	Good	Good	No	No	No	No	7	24		cover	3,500
M-25	1	119	19th St	06-030		Pavement - Concrete Collar			Good			Fair	Poor	No		No	No				Line manhole	10,000
M-26	1		Blanche Rd	17-040		Pavement - Concrete Collar			Good			Good	Poor	No			No				Line manhole	10,000
	1																	1				
M-27	1	26	1st St	01-061	11/21/08	Pavement - Concrete Collar	Good	Good	Good	Good	Fair	Good	Poor	No	No	0	No	7	27		Line manhole	10,000
M-28	1	143	Marine Ave	15-053	1/7/09	Pavement - Concrete Collar	Good	Good	Poor	Good	Fair	Good	Good	No	No	No	No	7	28	Missing mortar and possibly bricks.	Repair mortar	6,000
M-29	1	87	Highview Ave	11-040	1	Pavement - Concrete Collar			Good			Good	Good	Yes			No	7			Line manhole	10,000
M-30	2	191	1st St	12-026		Pavement - Concrete Collar			Good			Poor	Fair	No			No	7			Line manhole	10,000
							1		1	1		1									Replace manhole	
M-31	2	278	3rd St	10-139	7/22/09	Pavement - Concrete Collar	Cracked	Good	Good	Good	Good	Good	Good	No	No	No	No	7	31		cover	3,500
M-32	1	100	Manhattan Ave	11-117		Pavement - Concrete Collar		Good					Good	No		No	No	6			Line manhole	10,000
																					Total	I 316,000

Table 1-6

		r ump	Station and Force	•							
		Date of		Re	commend	led Faciliti	es				
Project No.	Project Description	Construction for Existing Facility	Justification	Firm Capacity (gpm)	Volume (gal)	Pipe Size (in)	Pipe Length (ft)	Unit Cost (\$)	Construction Cost (\$)	Eng. & Admin. Cost (\$)	Total Project Cost (\$)
P-1	Replace Poinsettia PS	1949	Condition / Criteria	150					2,000,000	700,000	2,700,000
	Replace Poinsettia PS Forcemain	1949	Age / Condition			4	163	300	49,000	18,000	67,000
P-2	Replace Pier PS Forcemain	1935	Age / Condition			4	900	400	360,000	126,000	486,000
P-3	Upgrade Pacific PS	1953	Criteria	400					400,000	140,000	540,000
	Replace Pacific PS Forcemain	1953	Age / Condition			6	1,225	240	294,000	102,900	396,900
P-4	Upgrade Voorhees PS	1953	Criteria	350					400,000	140,000	540,000
	Replace Voorhees PS Forcemain	1953	Age / Condition			6	930	240	223,200	78,120	301,320
P-5	Upgrade Meadows PS	1953	Criteria	310					400,000	140,000	540,000
	Replace Meadows PS Forcemain	1953	Age / Condition			6	730	240	175,200	61,320	236,520
P-6	Upgrade Bell Pump Station	1938	Criteria	300					400,000	140,000	540,000
	Replace Bell PS Forcemain	1938	Age / Condition			6	900	240	216,000	75,600	291,600
P-7	Replace Palm PS Forcemain		Age / Condition			4	775	240	186,000	65,100	251,100
P-8	Construct Emergency Storage for Pacific PS	-	Criteria		12,000			70	840,000	294,000	1,134,000
P-9	Construct Emergency Storage for Voorhees PS	-	Criteria		10,500			70	735,000	257,250	992,250
P-10	Construct Emergency Storage for Meadows PS	-	Criteria		9,300			70	651,000	227,850	878,850
P-11	Construct Emergency Storage for Bell PS	-	Criteria		8,400			70	588,000	205,800	793,800
P-12	Construct Emergency Storage for Palm PS	-	Criteria		4,800			70	336,000	117,600	453,600
								Total	8,253,400	2,889,540	11,142,940

 Table 1-7

 Pump Station and Force Main Improvement Projects

CIP No.	Project Description	Date of Construction for Existing Facility	Justification	water Capital Improvement Program Recommended Facilities				ſ			
				Firm Capacity (gpm)		Pipe Size (in)	Pipe Length (ft)	Unit Cost (\$)	Construction Cost (\$)	Eng. & Admin. Cost (\$)	Total Project Cost (\$)
1	Replace Poinsettia PS	1949	Condition / Criteria	150					2,000,000	700,000	2,700,000
	Replace Poinsettia PS Forcemain	1949	Age / Condition			4	163	300	49,000	18,000	67,000
2	Replace Pier PS Forcemain	1935	Age / Condition			4	900	400	360,000	126,000	486,000
3	Upgrade Pacific PS	1953	Criteria	400					400,000	140,000	540,000
	Replace Pacific PS Forcemain	1953	Age / Condition			6	1,225	240	294,000	102,900	396,900
4	Upgrade Voorhees PS	1953	Criteria	350					400,000	140,000	540,000
	Replace Voorhees PS Forcemain	1953	Age / Condition			6	930	240	223,200	78,120	301,320
5	Upgrade Meadows PS	1953	Criteria	310					400,000	140,000	540,000
	Replace Meadows PS Forcemain	1953	Age / Condition			6	730	240	175,200	61,320	236,520
6	Upgrade Bell Pump Station	1938	Criteria	300					400,000	140,000	540,000
	Replace Bell PS Forcemain	1938	Age / Condition			6	900	240	216,000	75,600	291,600
7	Replace Palm PS Forcemain		Age / Condition			4	775	240	186,000	65,100	251,100
8	Construct Emergency Storage for Pacific PS	-	Criteria		12,000			70	840,000	294,000	1,134,000
9	Construct Emergency Storage for Voorhees PS	-	Criteria		10,500			70	735,000	257,250	992,250
10	Construct Emergency Storage for Meadows PS	-	Criteria		9,300			70	651,000	227,850	878,850
11	Construct Emergency Storage for Bell PS	-	Criteria		8,400			70	588,000	205,800	793,800
12	Construct Emergency Storage for Palm PS	-	Criteria		4,800			70	336,000	117,600	453,600
*13	Gravity Sewer Rehabilitation and Replacement	Varies	Condition								28,930,000
*14	Manhole Rehabilitation and Replacement	Varies	Condition								596,000
**15	Collection System Capacity Deficiencies	Varies	Criteria			12 & 15	1,725				1,373,700
	* Total project cost includes a pro							Total	8,253,400	2,889,540	42,042,640

Table 1-8 Vastewater Capital Improvement Program